Updates to the MODFLOW Groundwater Model of the San Antonio Segment of the Edwards Aquifer

Presented by
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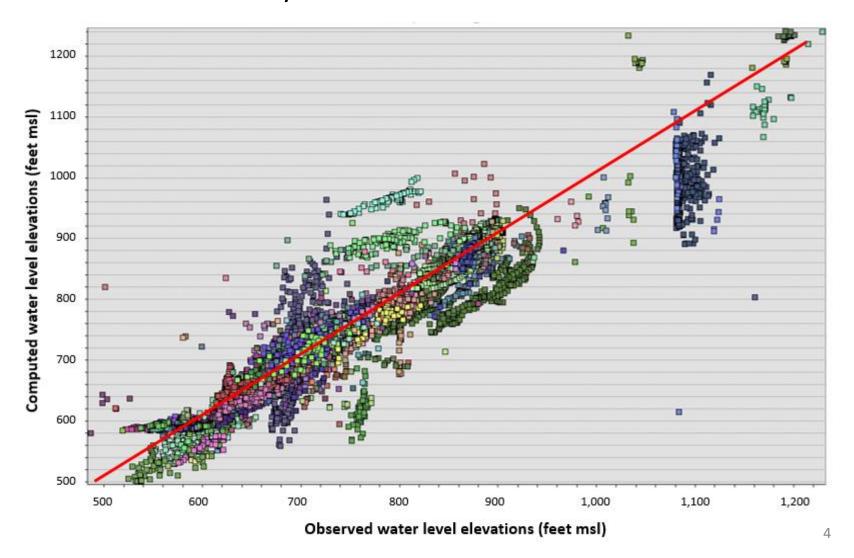
Background

- The goal of the MODFLOW model in the Phase II EAHCP process is to examine spring flow protection measures and inform any refinements that need to be made to those measures
- The EAA model represents a multi-year update of a 2004 model produced by Lindgren et al. (2004)
- The EAA completed updates and solicited feedback from a Groundwater Model Advisory Panel (GMAP) in March 2017
- A report documenting parameterization, calibration, and validation was also reviewed by GMAP members and ready for publication

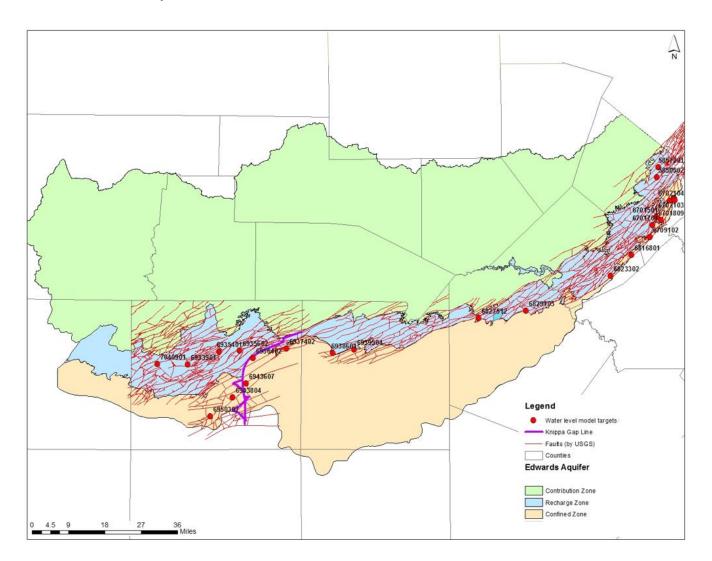
In-Depth Review of Original MODFLOW Model: "Functionality and Verification Analyses"

- Original model calibrated to data from period 1947—2000
- <u>Functionality Analysis</u> conducted to compare original model simulation results to observation data not used in the original calibration
- Verification Analysis ran original model forward for years 2001—2009 to evaluate how well model predicts water levels and spring flows for a period it was not calibrated to
- Results of these analyses informed model updates

Simulated vs. Observed Water Levels for Functionality Test of 2004 Model



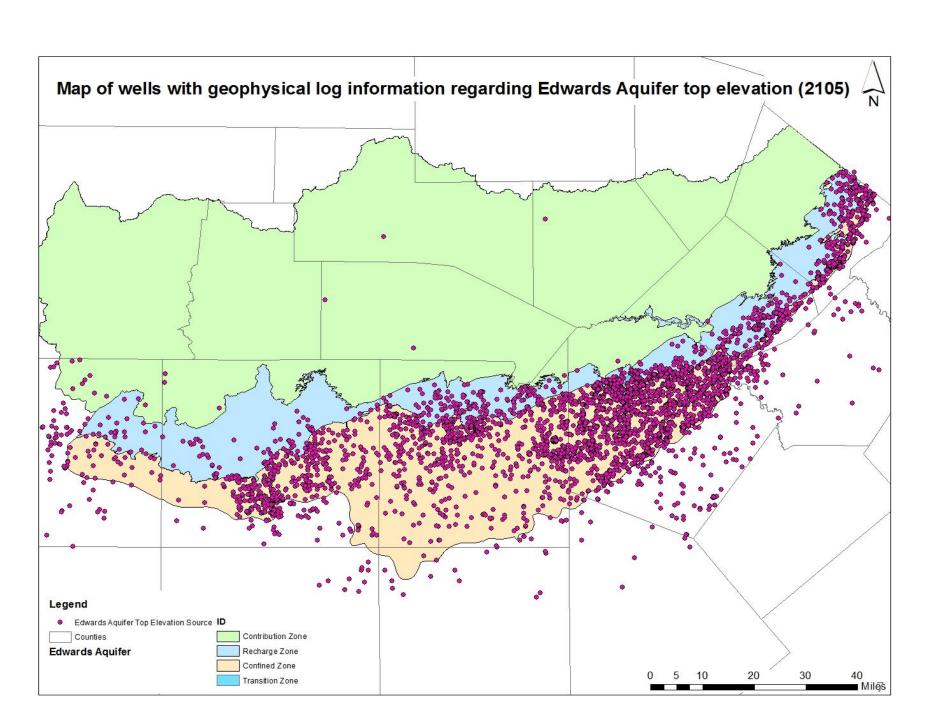
Locations of Wells with Largest Errors in Functionality Test

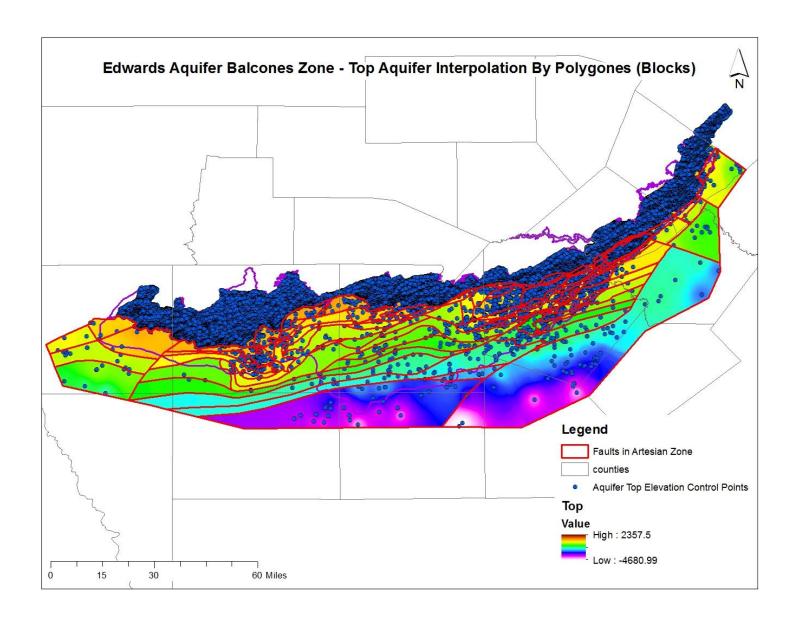


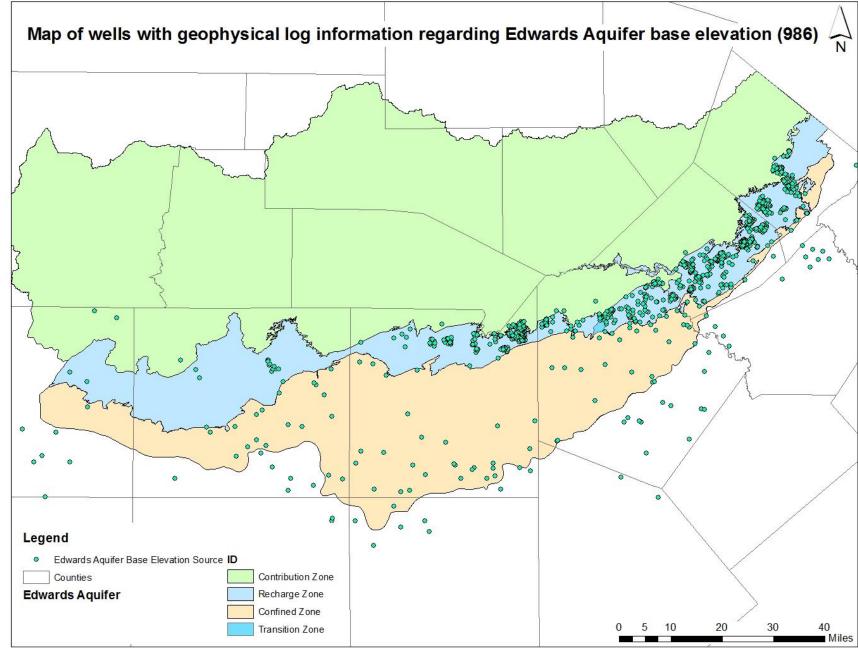
Model Update and Recalibration

Major changes from original model include:

- Removed Barton Spring segment
- New tops and bottom layer elevation model
- Hydraulic conductivity zones modified to remove explicit conduits
- Added HFB flow barriers to represent Knippa Gap and Haby's Crossing fault zone
- Known locations of wells and annual pumping totals
- Added two new spring locations to represent Hueco Springs and subsurface discharge in Leona River basin
- Increase rate of interformational flow in norther Bexar County

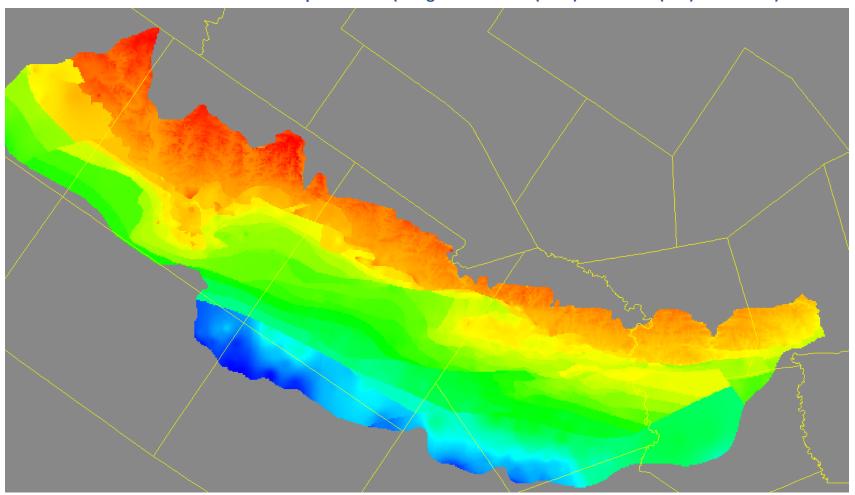






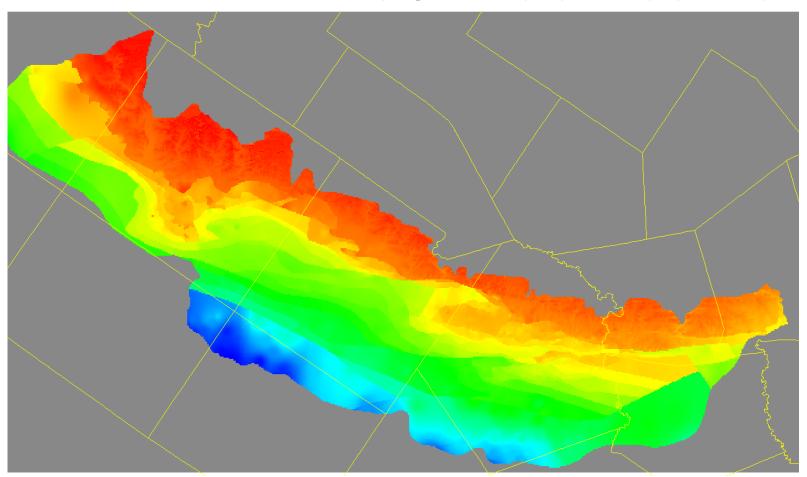
Updated Model Top Elevation

Contour of Simulated Model Top Elevation (Range from -4064 (blue) to 2018 ft (red) above msl)



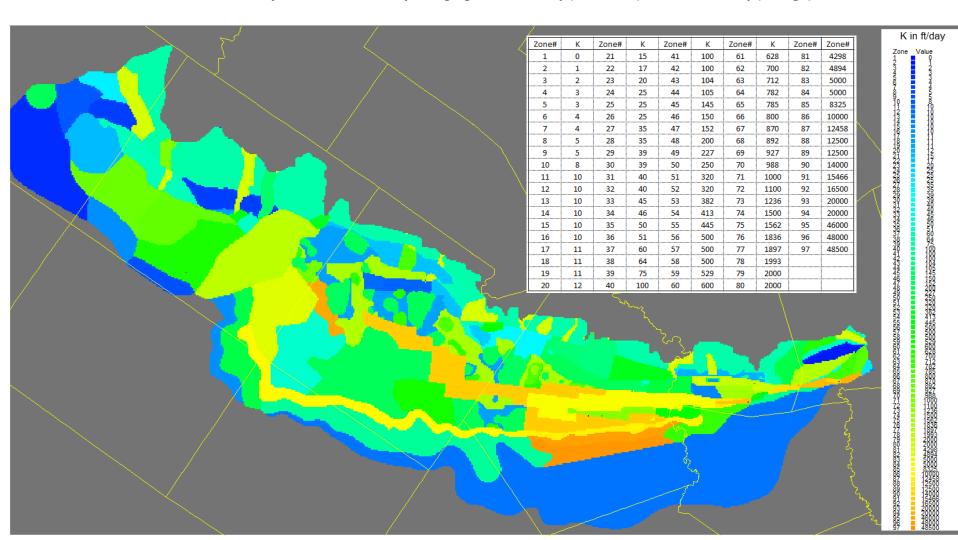
Updated Model Bottom Elevation

Contour of Simulated Model Bottom Elevation (Range from -5000 (blue) to 1114 ft (red) above msl)



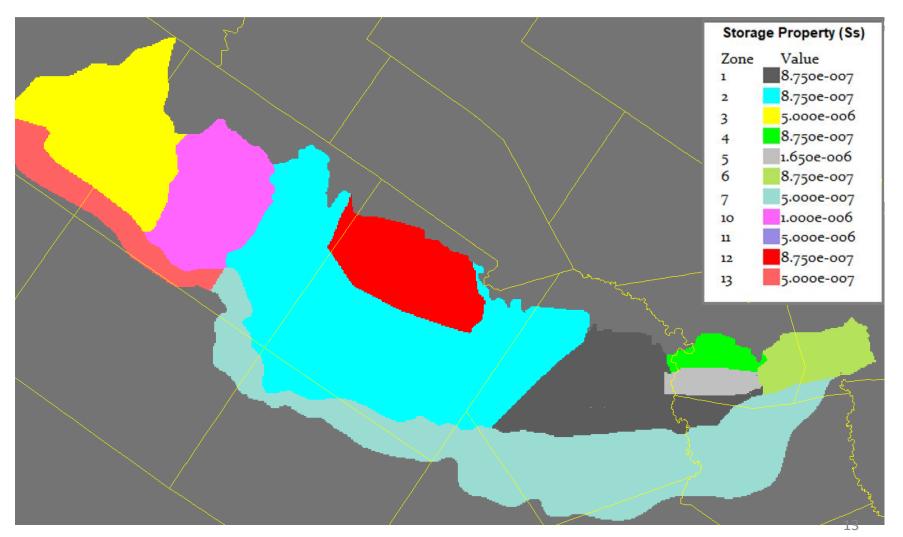
Hydraulic Conductivity Zones

Simulated Distribution of Hydraulic Conductivity, Ranging from 1 ft/day (dark blue) to 48,500 ft/day (orange)



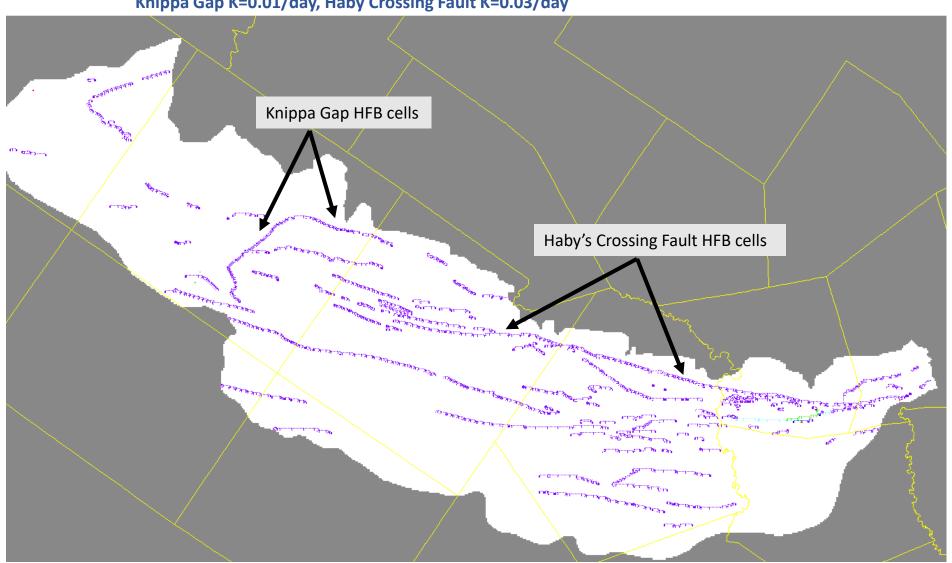
Storage Parameter Zones

Specific Storage (Ss) 5.00E-06 to 5.00E-07 d⁻¹ (same as original 2004 model) Specific Yield (Sy) 1.00E-03 to 2.12E-01 (5.00E-03 to 1.50E-01 in 2004 model)

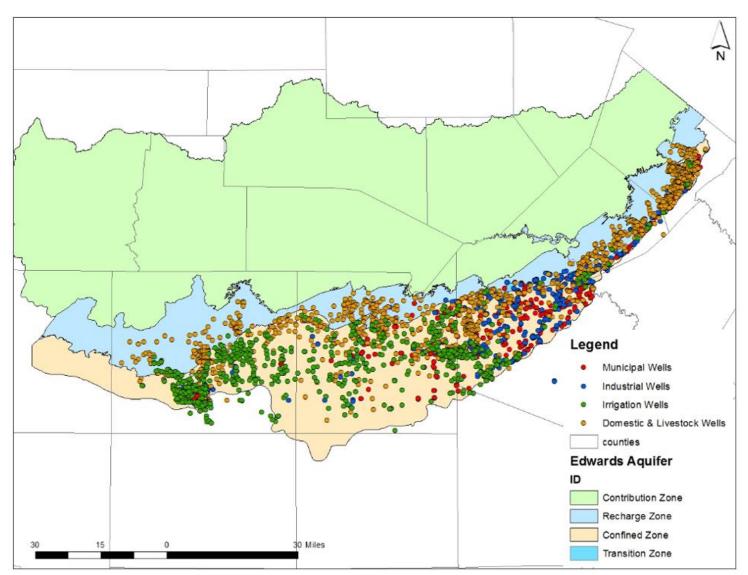


Horizontal Flow Barriers

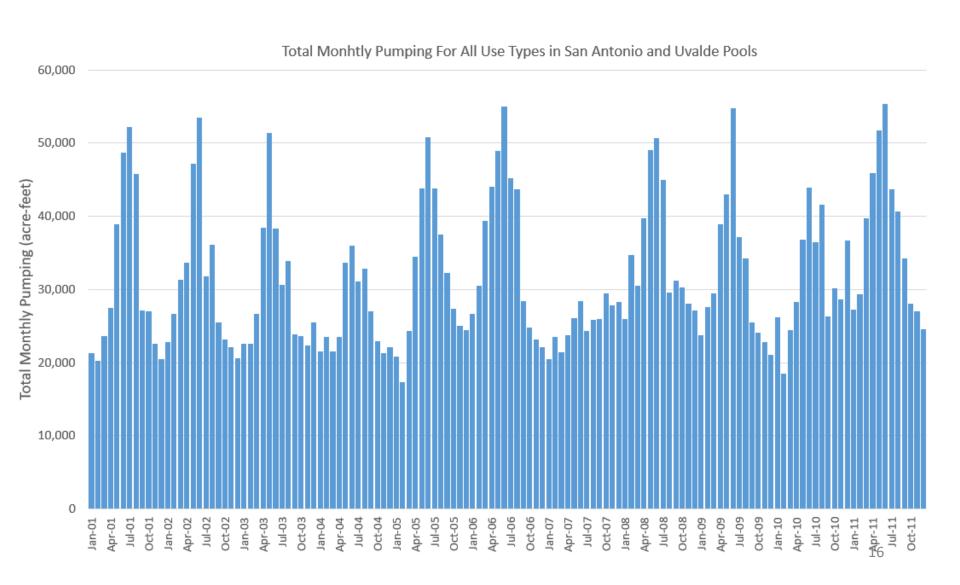
Knippa Gap K=0.01/day, Haby Crossing Fault K=0.03/day



Pumping Locations and Type of Use

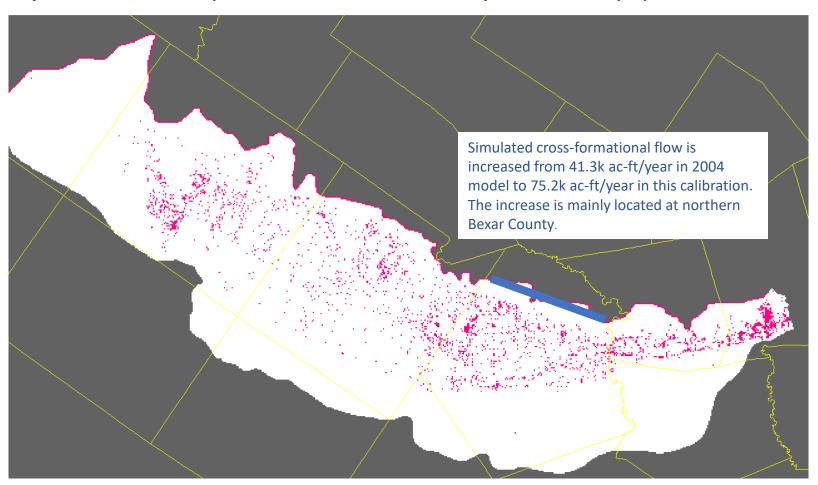


Monthly Pumping Rates Estimated for Each Location

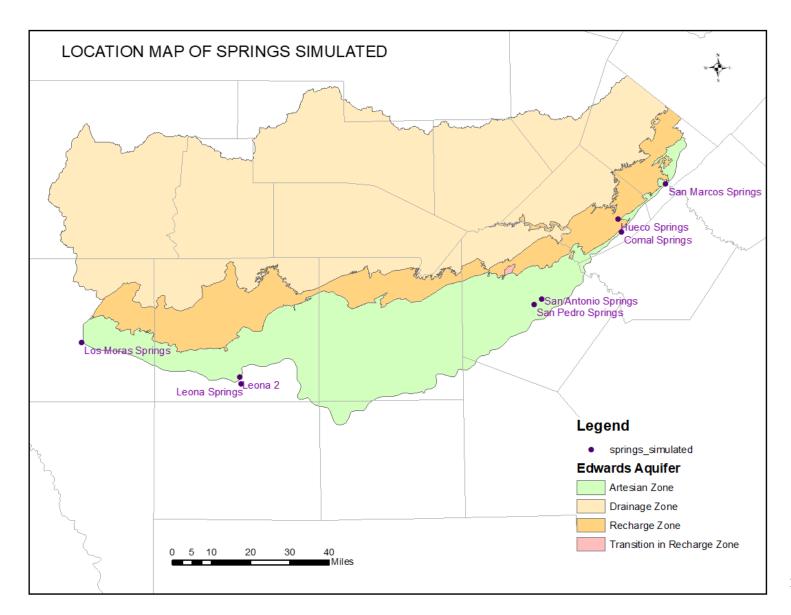


Pumping well locations

Injection wells used to represent cross-formational boundary flow from Trinity aquifer to the north



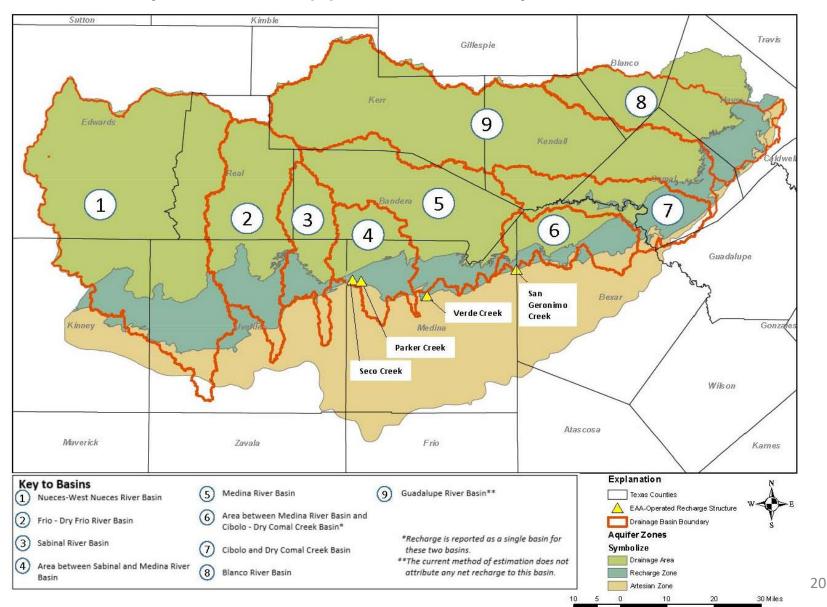
Modeled Springflow Locations



Modifications to USGS recharge estimates based on same method developed by Lindgren et al. for the original model

- Recharge to the Cibolo and Dry Comal Creek watershed area is reduced by a factor of 0.5 for all monthly stress periods
- In years when the USGS aquifer-wide total annual recharge estimate exceeds 1.4 million acre-feet, recharge to all basins is multiplied by a factor of 0.8 for all stress periods during that year, after applying the above corrections. In the updated model, this reduction was applied to years 2002, 2004, and 2007
- Recharge to Nueces-West Nueces River watershed was increased by a factor of 1.048 for all monthly stress periods
- Recharge to Frio Dry Frio watershed area was increased by a factor of 1.011 for all monthly stress periods.

Recharge estimates start with USGS estimates for 8 watershed areas Guadalupe watershed (9) not estimated by USGS

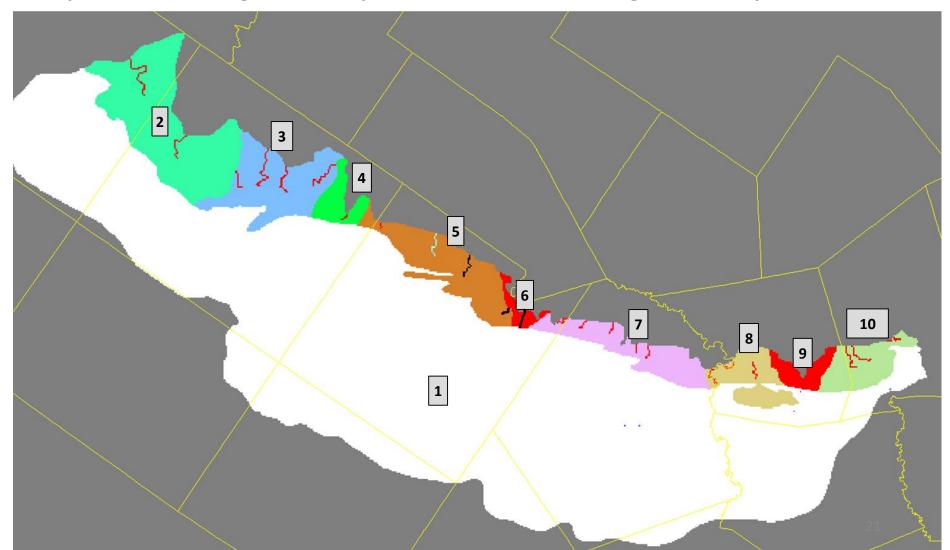


Distribution of Recharge

15% assigned as distributed recharge in nine watershed zones

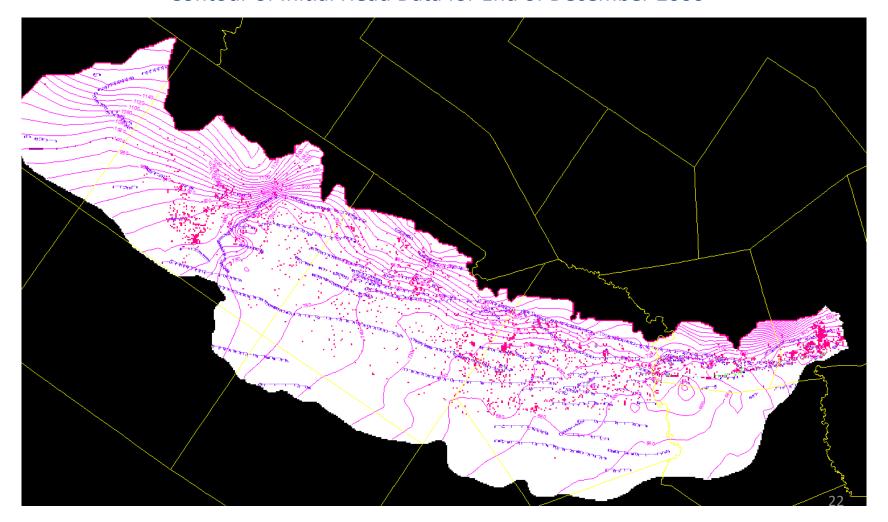
85% assigned to 23 stream segments

Only distributed recharge in Guadalupe River basin, based on average rates for adjacent basins



Initial Condition for Hydraulic Heads

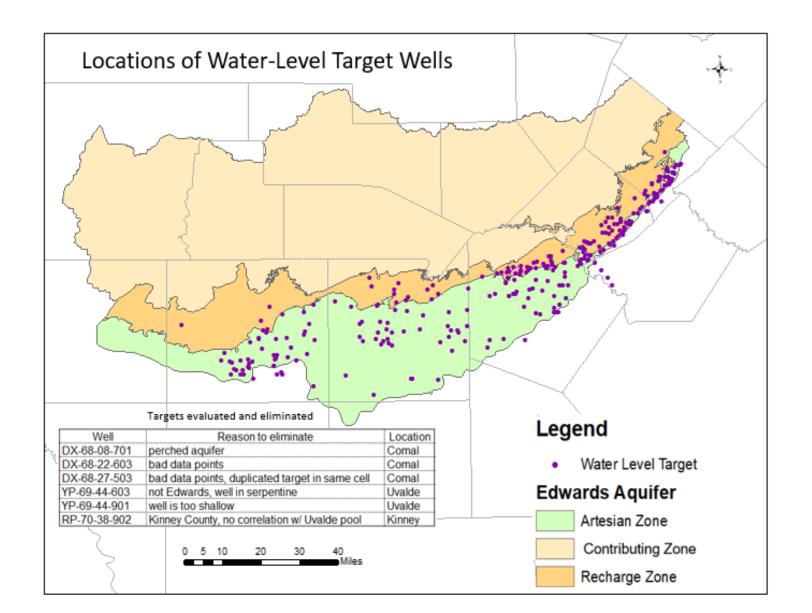
Contour of Initial Head Data for End of December 2000



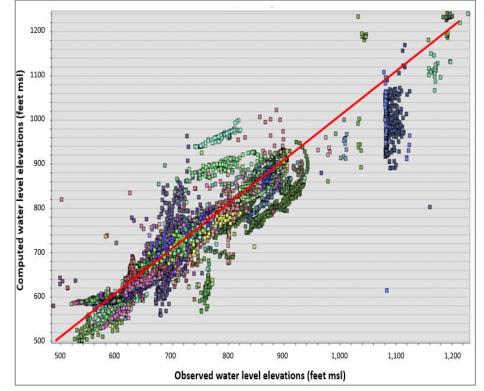
Model Calibration

- Parameters varied during calibration:
 - The shape, and assigned hydraulic conductivity of 96 delineated hydraulic conductivity zones
 - Specific yield of the 12 storage zones
 - Hydraulic characteristic parameter for the HFB locations representing the Haby's Crossing fault and Knippa Gap area
 - Drain elevation and conductance parameters for the drain cells representing spring discharge locations, and
 - Boundary inflow rates representing interformational flow across the northern model boundary
- January 2001 through December 2011 calibration period—132 monthly time steps

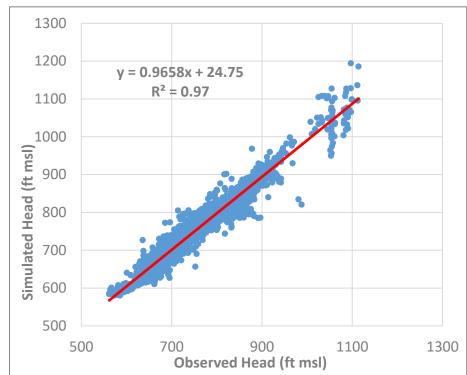
Water-Level Observation Locations

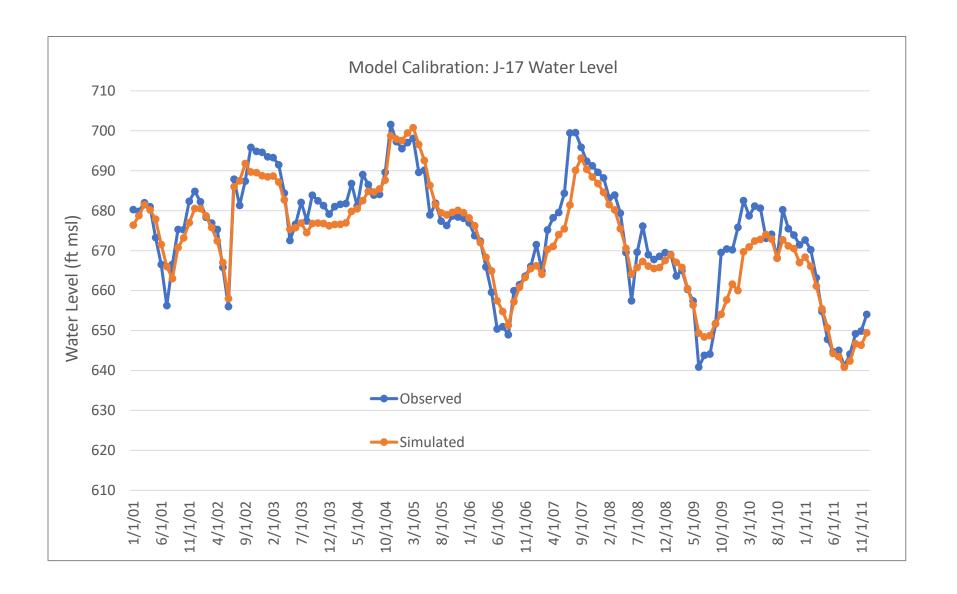


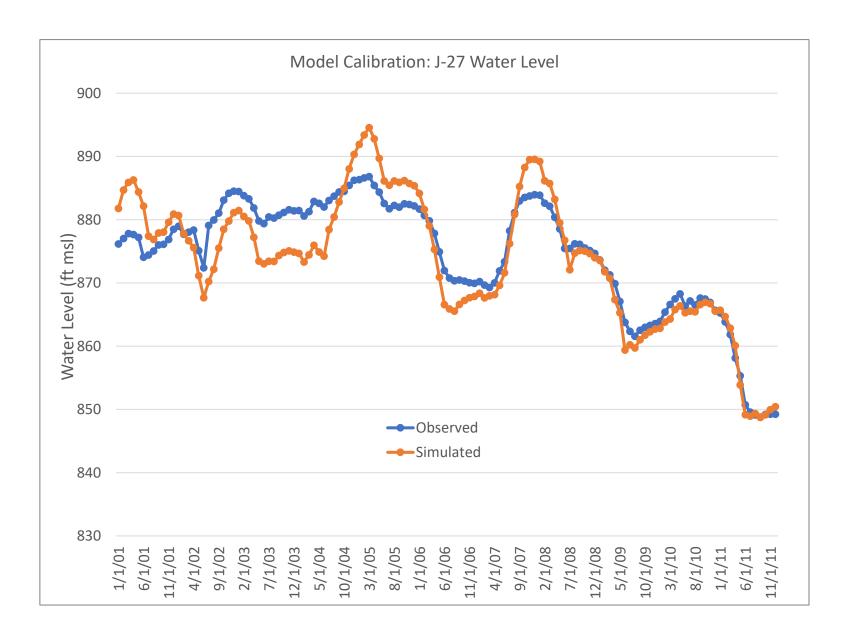
Original Model 1947-2000

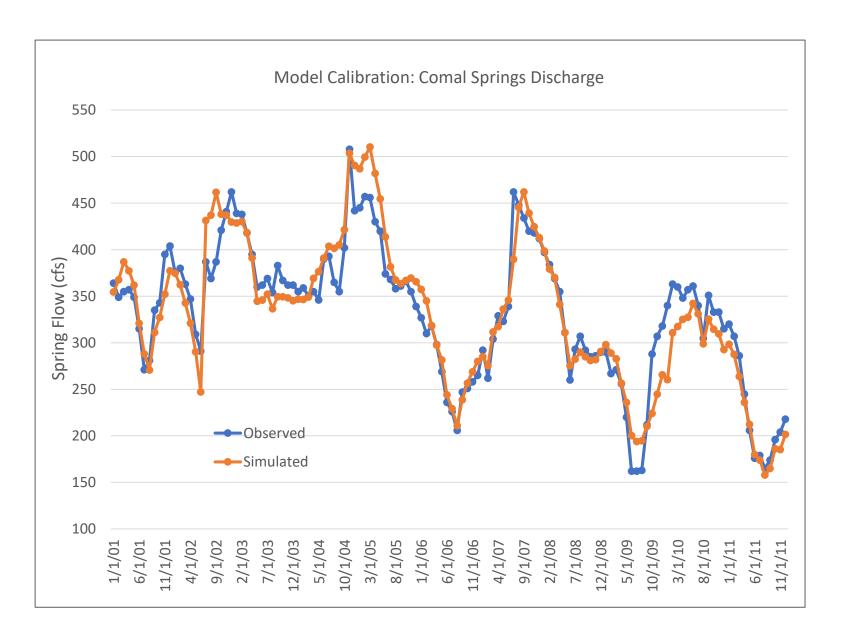


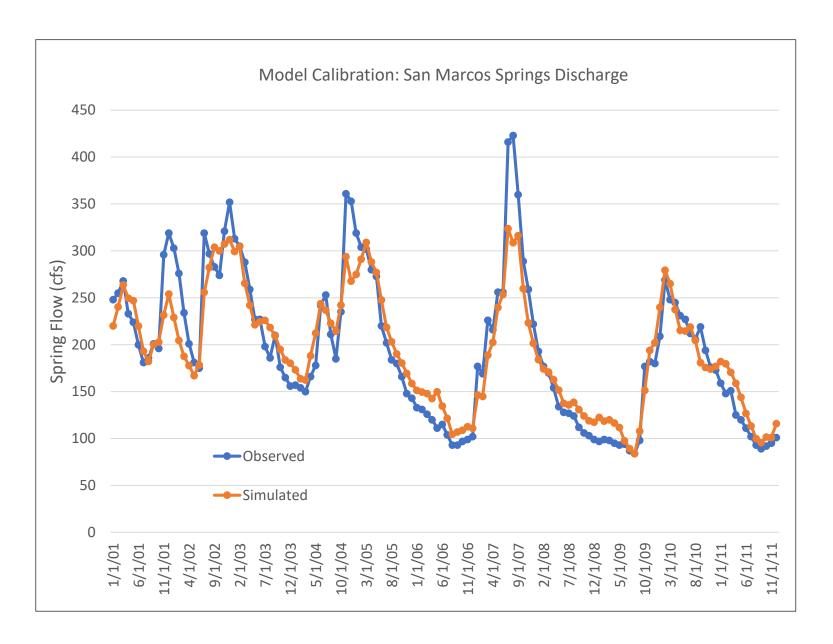
Updated Model 2001-2011

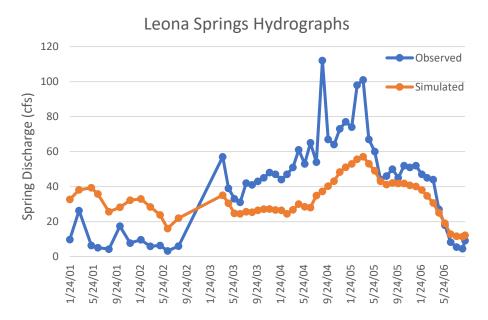


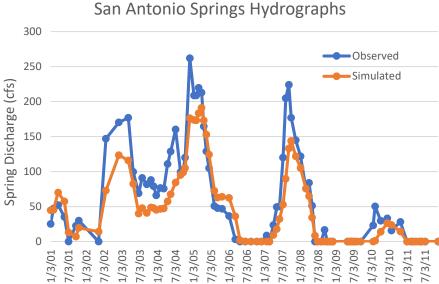


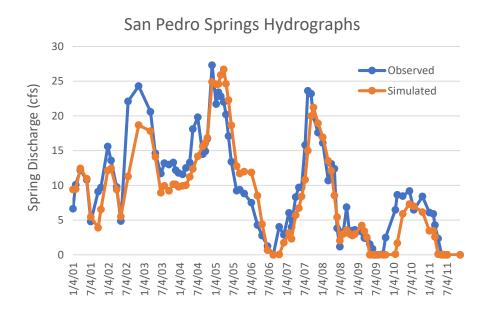


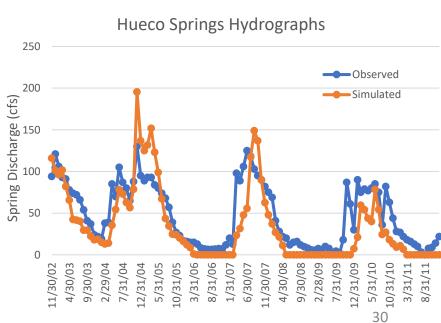












HYDRAULIC HEAD CALIBRATION STATISTICS

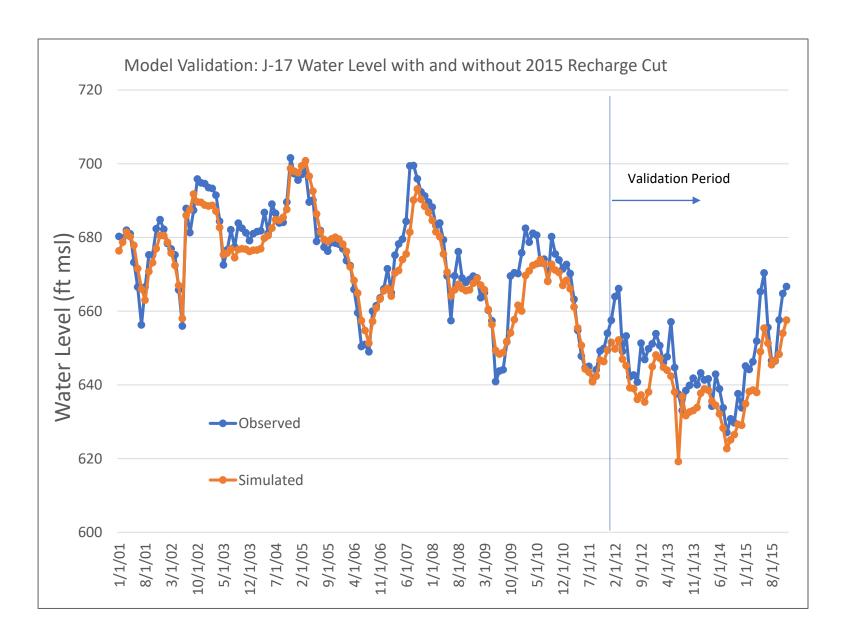
Error Statistic	Proposed Criterion	Original 2004 Model	Updated Model
Mean Error, all observations	≤ 2.0 ft	-14.4 ft	-0.45 ft
Mean Absolute Error, all observations	≤ 20 ft	25.7 ft	11.7 ft
Root-Mean-Square (RMS) Error, all observations	≤ 25 ft	38.4 ft	17.0 ft
RMS-Error to Range-of-Observations Ratio	≤ 10%	5.1%	3.1%
J-17 Mean Error	≤ 2.0 ft	3.9 ft	1.9 ft
J-17 RMS Error	≤ 7.0 ft	7.9 ft	5.0 ft
J-17 Maximum Absolute Error	≤ 30 ft	10.3 ft	18 ft
J-27 Mean Error	≤ 1.3 ft	-31.0 ft	0.7 ft
J-27 RMS Error	≤ 5.0 ft	30.7 ft	4.0 ft
J-27 Maximum Absolute Error	≤ 20 ft	46.8 ft	8.9 ft

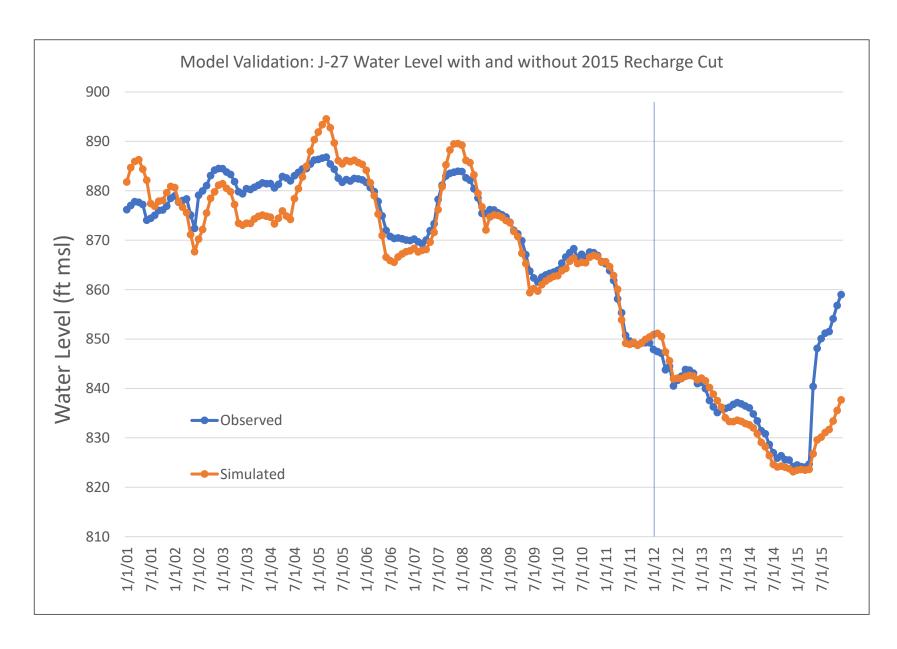
SPRING TARGETS CALIBRATION SUMMARY

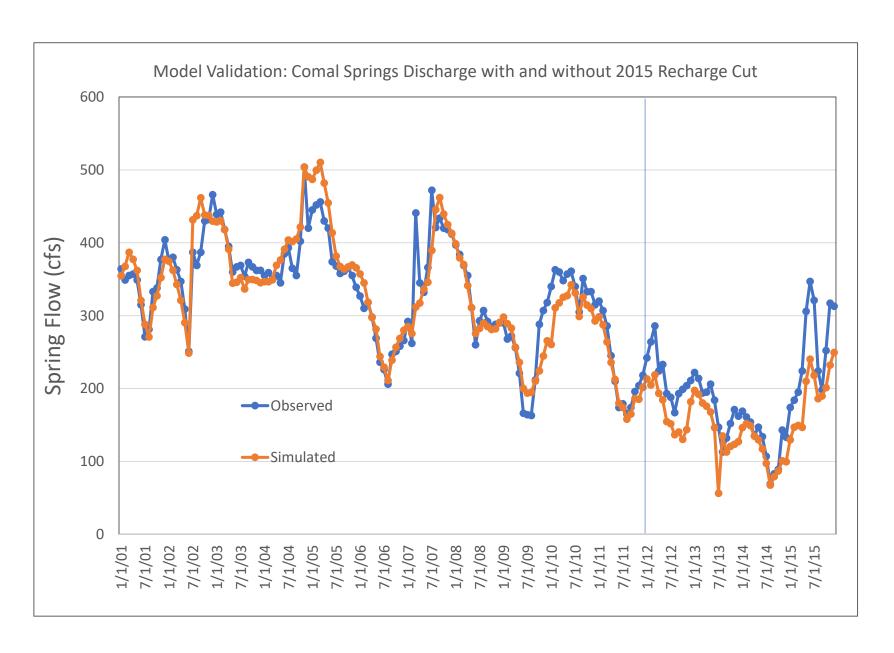
Error Statistic	Proposed Criterion	Original 2004 Model	Updated Model
Comal Springs Mean Error	≤ 3.0 cfs	14.9 cfs	0.4 cfs
Comal Springs RMS Error	≤ 50 cfs	37.9 cfs	26.2 cfs
Comal Springs Cumulative Error	≤ 3%	4.0 %	0.12%
Comal Springs Maximum Absolute Error	≤ 150 cfs	139 cfs	79.7 cfs
San Marcos Springs Mean Error	≤ 3 cfs	43.6 cfs	0.8 cfs
San Marcos Springs RMS Error	≤ 35 cfs	62 cfs	28.0 cfs
San Marcos Springs Cumulative Error	≤ 3%	22%	0.4%
San Marcos Springs Maximum Absolute Error	≤ 150 cfs	134 cfs	114.3 cfs

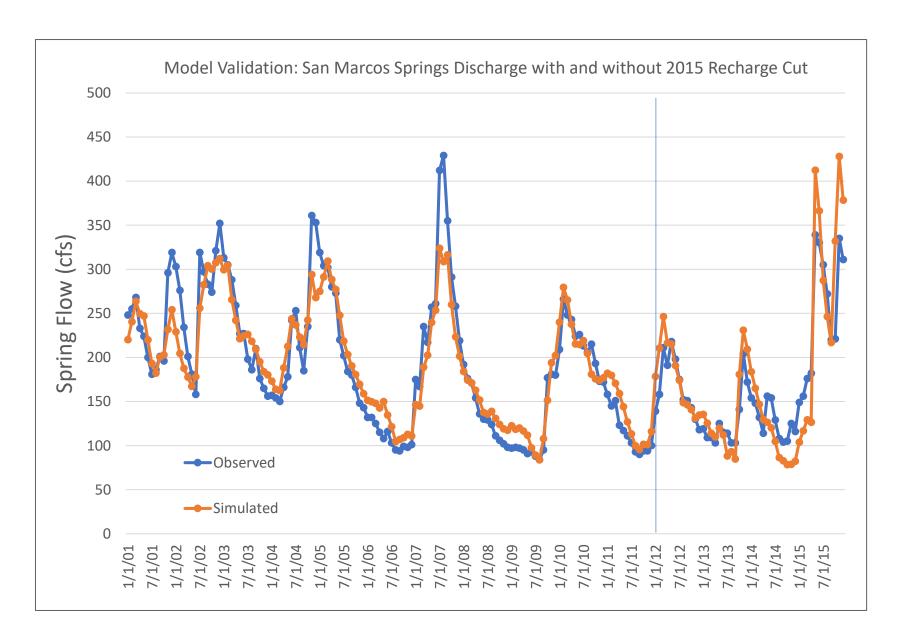
Model Validation Test

- Model run forward for a period that was not used in the calibration: January 2012— December 2015
- 48 additional monthly time steps
- Specifically suggested in NAS 2
- Includes lowest water levels observed during the 2008—2014 drought and recovery from drought in 2015



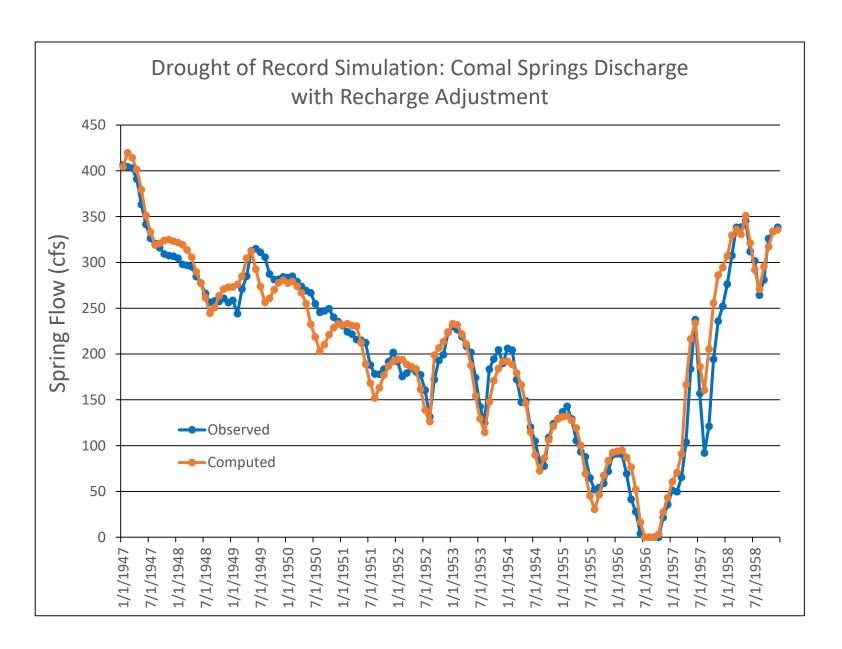


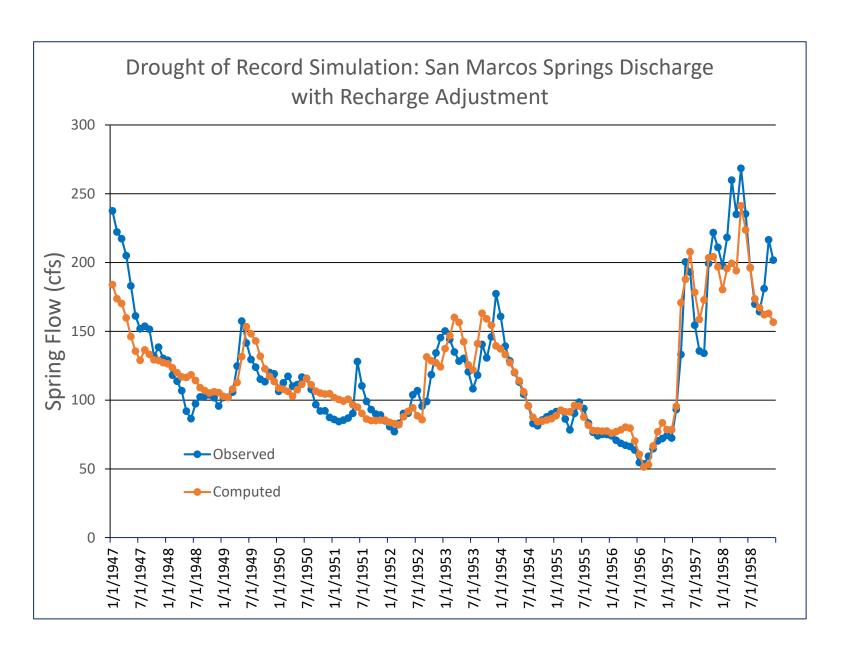




Drought-of-Record Simulations

- Set up drought-of-record scenario using pumping and recharge estimates for the period of January 1947 through December 1958 to use for HCP Adaptive Management evaluations
- Model goal for DOR scenario is to closely match observed spring flows—especially minimum flows observed during 1956
- No changes made to calibrated model parameters
- Small adjustment made to recharge near San Marcos springs to better match observed spring flows

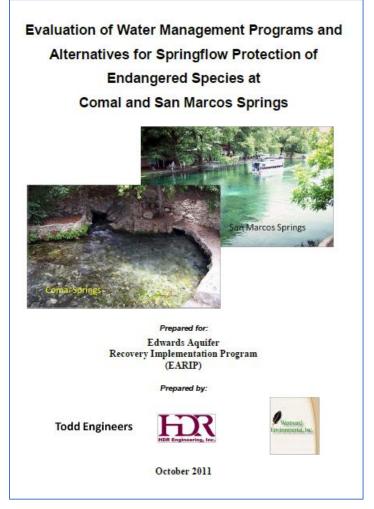




Repeat HDR (2011) "Bottom-Up Analysis with Updated Model

 Original 2004 MODFLOW Model used to demonstrate effectiveness of conservation measures to preserve spring flows

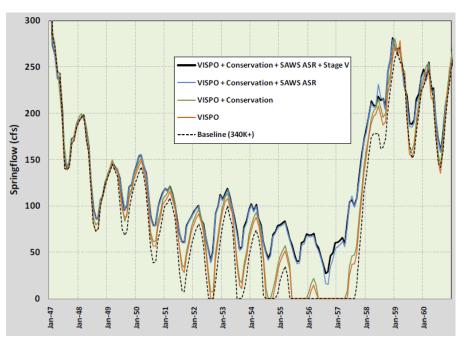
1	Layer 4	Layer 3 + Stage V Emergency Reductions
BOTTOM-UP Program	Layer 3	Layer 2 + SAWS ASR with Trade-Off Option
BOTTC Prog	Layer 2	Layer 1 + Municipal Conservation Measures
	Layer 1	Voluntary Irrigation Suspension Program Option



Source: HDR, Inc. (2011)

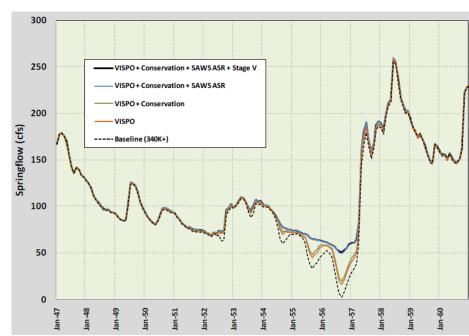
Original Bottom-Up Results

Comal Springs minimum flow: **27 cfs** in August 1956



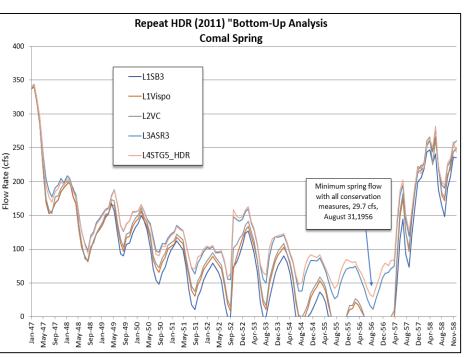
Source: HDR, Inc. (2011)

San Marcos Springs minimum flow: 51 cfs in August 1956

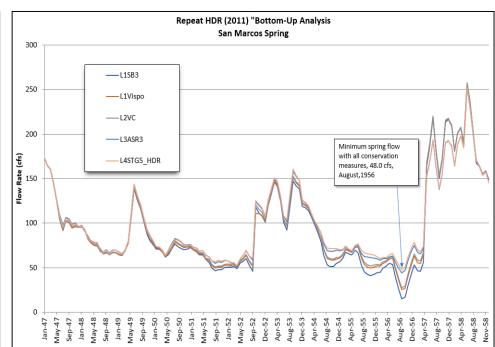


Updated model Bottom-Up Results

Comal Springs minimum flow: 29.7 cfs in August 1956



San Marcos Springs minimum flow: 48 cfs in August 1956



Next Steps

Publish Model Update Report by December 2017

- Use model to support HCP Adaptive Management
 - Repeat HDR (2011) bottom-up analysis with new assumptions regarding ASR leases and triggers

Next Steps

- Uncertainty analysis using PEST++ inverse parameter estimation software
 - Collaboration with USGS' Austin Office using their highperformance computer cluster and parallel processing methods
 - Evaluate uncertainty in hydraulic parameters and recharge quantity distribution
 - Simultaneous inversion to both the 2001—2015 period and the 1947—1958 drought-of-record period
 - Expected completion by March 2019

Questions?