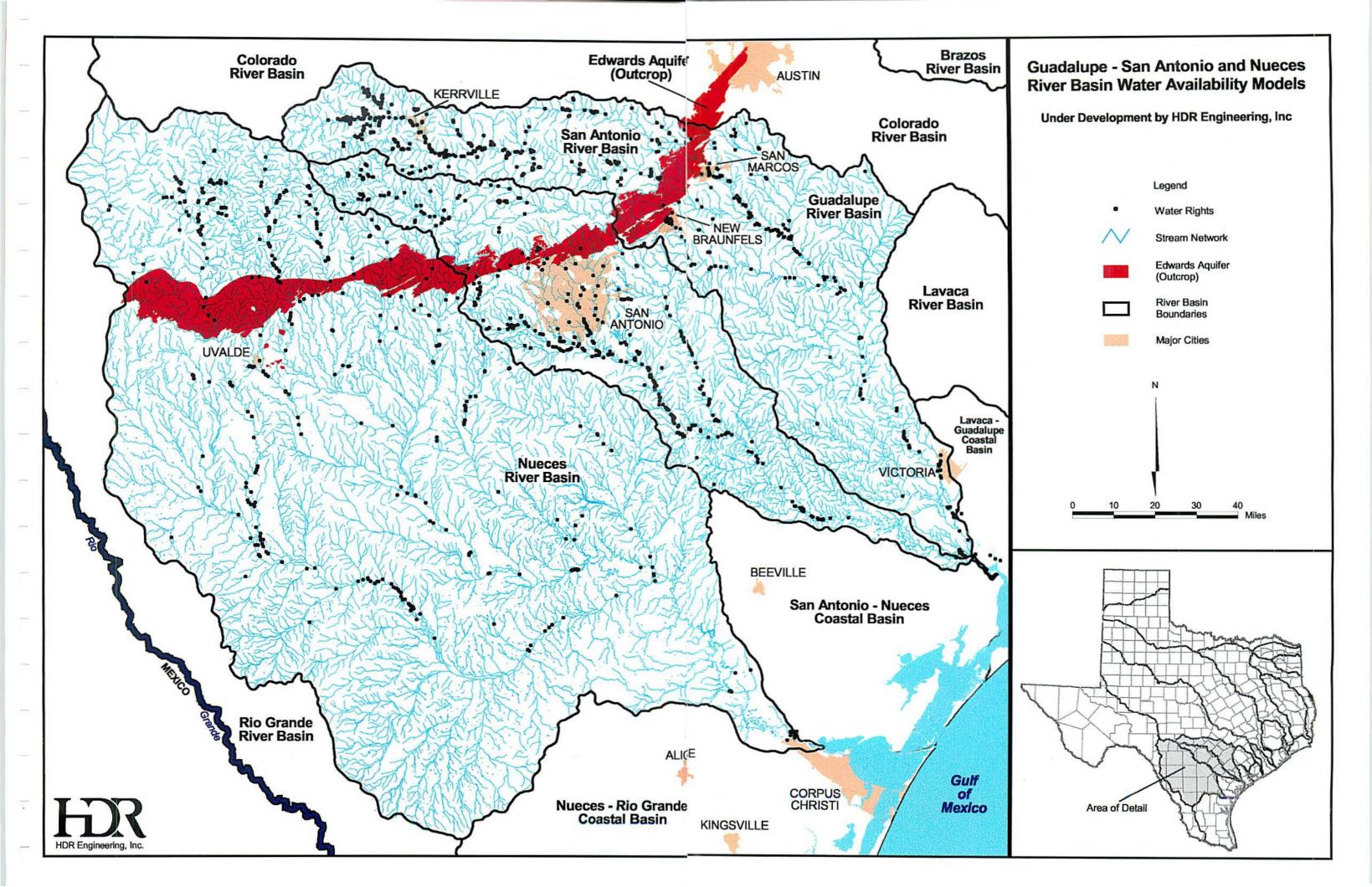
Edwards Aquifer Authority

Summary Information Regarding Historical Edwards Aquifer Recharge, Aquifer Modeling, and Recharge Enhancement Projects

> HDR Engineering, Inc. July 2, 1999



Historical Edwards Aquifer Recharge

1

1

-

T

-

-

-

-

[

ſ

I

[

-

TRANS-TEXAS WATER PROGRAM WEST CENTRAL STUDY AREA

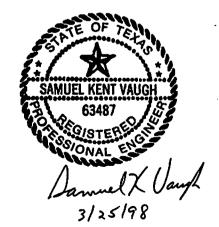
PHASE 2

EDWARDS AQUIFER RECHARGE UPDATE

San Antonio River Authority San Antonio Water System Edwards Aquifer Authority Guadalupe-Blanco River Authority Lower Colorado River Authority Bexar Metropolitan Water District Nueces River Authority Canyon Lake Water Supply Corporation Bexar-Medina-Atascosa Counties WCID No. 1 Texas Natural Resource Conservation Commission Texas Parks and Wildlife Department Texas Water Development Board



March 1998



TRANS-TEXAS WATER PROGRAM WEST CENTRAL STUDY AREA

EDWARDS AQUIFER RECHARGE UPDATE

TABLE OF CONTENTS

Secti	on		Page
1.0	INTI	RODUCTION	1-1
2.0	DAT	A COLLECTION AND REFINEMENT	2-1
3.0	REC	HARGE SUMMARY AND COMPARISONS	3-1
	3.1	Nueces River Basin	3-1
	3.2	Guadalupe - San Antonio River Basin	3-3
	3.3	General Comparisons	
4.0	REC	OMMENDATIONS	4-1

APPENDICES

Α	HISTORICAL EDWARDS AQUIFER RECHARGE ESTIMATES	A-	1
---	---	----	---

LIST OF FIGURES

.

Figuro No.	e	Page
1.0-1	Nueces River Basin Edwards Aquifer Recharge Basins	1-3

1.0-2	GSA River Basin Edwards Aquifer Recharge Basins	1-4
2.0-1	Reported Water Use Comparisons	2-2
3.1-1	Annual Edwards Aquifer Recharge Comparisons Nueces River Basin	3-2
3.2-1	Annual Edwards Aquifer Recharge Comparisons GSA River Basin	3-4
3.3-1	Geographical Comparison of Edwards Aquifer Recharge by Recharge Basin	3-7
3.3-2	Long Term Edwards Aquifer Recharge Comparisons	3-8
3.3-3	Geographical Comparison of Edwards Aquifer Recharge by River Basin	. 3-10

LIST OF TABLES

Table	
No.	Page

.

1.0 INTRODUCTION AND BACKGROUND

The 1990-96 historical period was one of extremes with respect to fluctuations in pumpage, water levels, and springflows associated with the Edwards Aquifer. Coming out of a drought in the late 1980's which resulted in record high annual pumpage (543,000 acft) in 1989, the Edwards Aquifer rose to a record high level of about 703 ft-msl recorded at the Bexar County Monitoring Well (J-17) in June, 1992 when pumpage fell to the lowest annual rate (327,000 acft) since 1973. Then, another drought cycle ensued resulting in significantly reduced springflows and severe water use restrictions during the summer of 1996. In addition to improved estimates of pumpage, the extremes experienced by the aquifer make the first half of the 1990's an excellent period for potential use in calibration of Edwards Aquifer models such as the GWSIM4 model developed by the Texas Water Development Board (TWDB).¹

The TWDB staff is, in fact, engaged in recalibration and enhancement of the GWSIM4 model which has been applied extensively in the Trans-Texas Water Program, Edwards Aquifer litigation, and numerous technical and planning studies. This recalibration effort has been prompted by the availability of improved geological mapping in Hays, Comal, and Bexar Counties, installation of a precipitation (and streamflow) gaging network in the Edwards outcrop area, completion of aquifer divide studies, and ongoing water balance studies for Medina Lake and the Guadalupe River. In addition, estimates of historical Edwards Aquifer recharge have been developed by HDR Engineering, Inc. (HDR) in the course of studies sponsored by the Edwards Underground Water District² and Nueces River Authority.³ Based on the 1934-89 historical period, HDR estimates differ significantly from those published by the U.S. Geological Survey⁴ (USGS) in terms of both geographical and temporal distribution.

As the TWDB has expressed an interest in using the most recent historical data available in the recalibration effort and regional sponsors have expressed their concurrence, HDR has

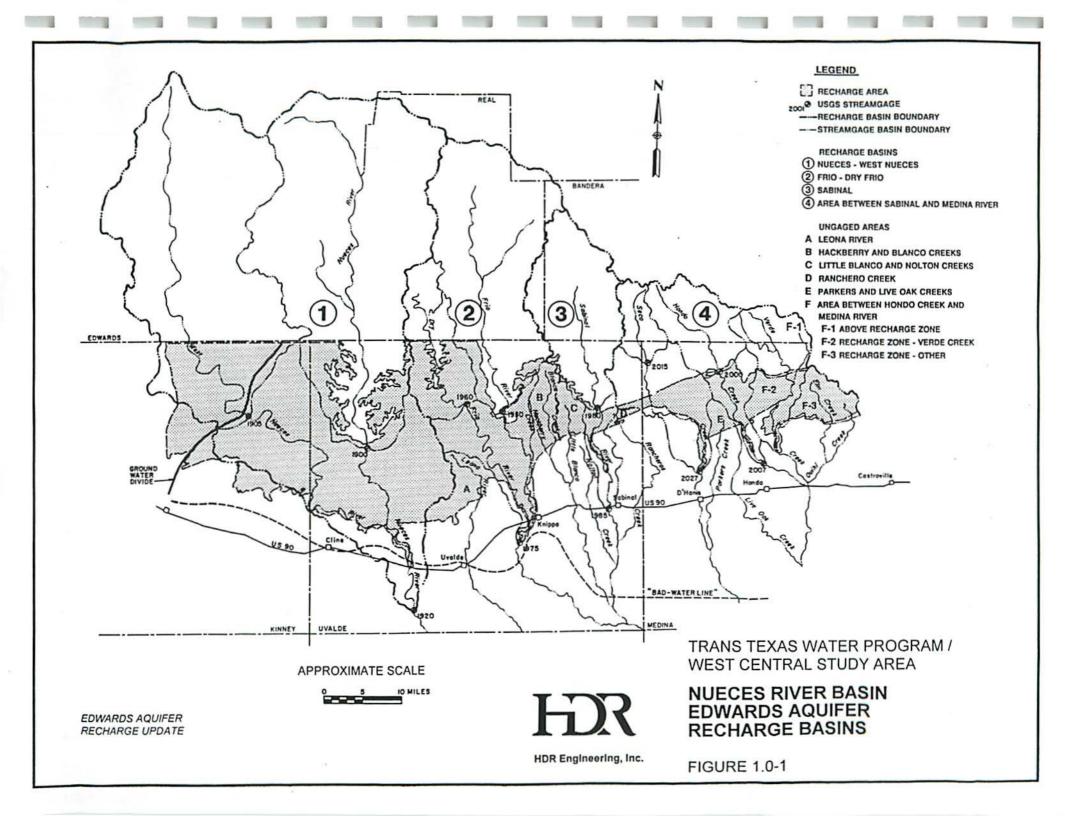
¹ TWDB, "Ground-water Resources and Model Applications for the Edwards (Balcones Fault Zone) Aquifer in the San Antonio Region," Report 239, October, 1979.

² HDR, "Guadalupe - San Antonio River Basin Recharge Enhancement Study," Vol. 2, Edwards Underground Water District, September, 1993.

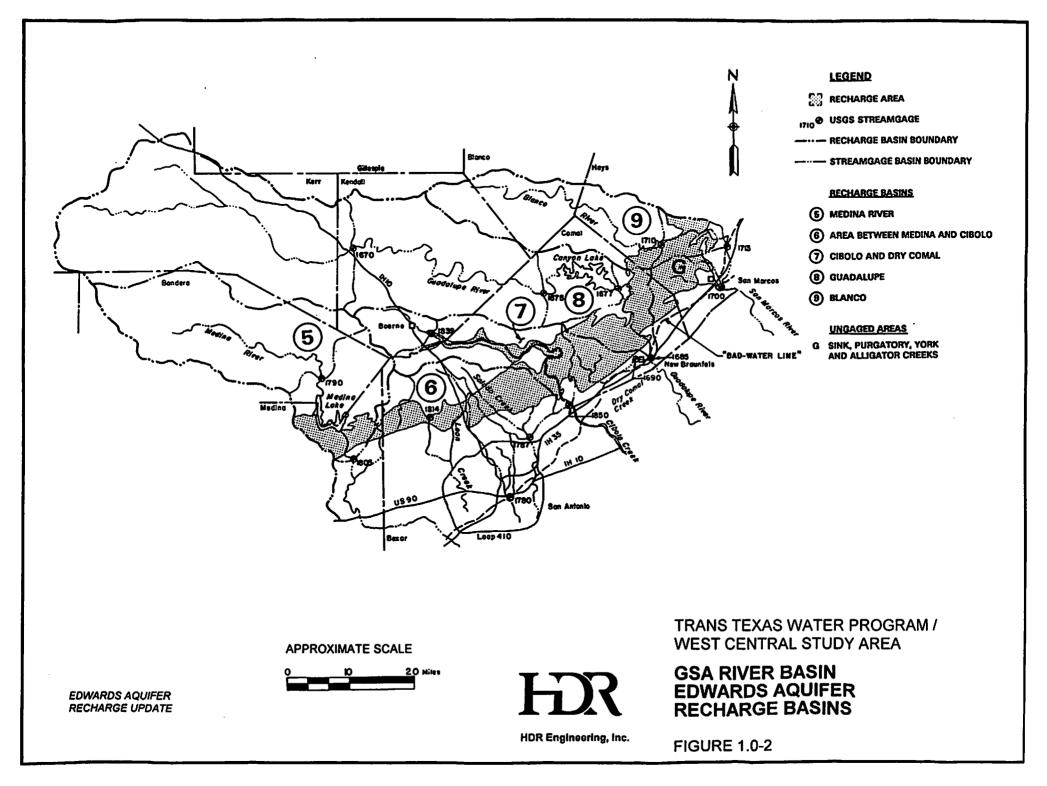
³ HDR, "Nueces River Basin Regional Water Supply Planning Study, Phase I," Vol. 2, Nueces River Authority, et al., May, 1991.

⁴ USGS, "Recharge to and Discharge from the Edwards Aquifer in the San Antonio Area, Texas, 1996," http://txwww.cr.usgs.gov/reports/info/97/recharge1/index.html, April, 1997.

updated its recharge estimates to include the 1990-96 historical period and will provide them to the TWDB for consideration as an alternative to published USGS estimates. Estimates of Edwards Aquifer recharge have been developed for four recharge basins in the Nueces River Basin (Figure 1.0-1) and five recharge basins in the Guadalupe - San Antonio River Basin (Figure 1.0-2) for the 1990-96 historical period. The following sections of this report detail the data collection and refinement efforts prerequisite to recharge calculation, summarize the resulting estimates of Edwards Aquifer recharge in both historical and geographical contexts, and provide comparisons to published USGS estimates. Recommendations regarding opportunities for improvement of recharge estimates are included in Section 4.







2.0 DATA COLLECTION AND REFINEMENT

The first step in the process of Edwards Aquifer recharge calculation was the collection of pertinent monthly hydrologic data sets including precipitation, streamflow, reservoir contents, surface water use, treated effluent volumes, and net evaporation for the 1990-96 historical period. Pertinent hydrologic data sets collected and primary sources are summarized as follows:

- Precipitation National Weather Service, USGS, TWDB
- Streamflow USGS
- Reservoir Contents USGS, Bexar-Medina-Atascosa Counties WCID#1 (BMA), Blackwell, Carter & Associates, Inc. (BCA)
- Surface Water Use Texas Natural Resource Conservation Commission (TNRCC, Office of the Water Master), USGS, BMA, BCA
- Treated Effluent Volumes TNRCC
- Net Evaporation BCA

Supplementary hydrologic data collected also includes monthly estimates of recharge for existing enhancement projects provided by the Edwards Aquifer Authority (EAA) and annual historical recharge by basin available from the USGS.

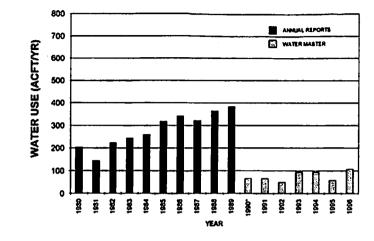
Once all pertinent information was in hand and prior to initiating recharge calculations, data sets from various sources were assembled and refined through review for consistency, estimation of unavailable data, areal precipitation computation, streamflow naturalization, and potential runoff calculation. Only one concern was noted regarding consistency of data for the 1990-96 period as compared with earlier years. This concern is associated with reported surface water use data provided by the TNRCC Water Master and its consistency with earlier data which was obtained from the TNRCC (prior to full implementation of the Water Master program). Figure 2.0-1 shows reported surface water use for four selected stream segments upstream of the Edwards Aquifer recharge zone for the 1980-96 period. While the apparent inconsistencies shown in Figure 2.0-1 may appear rather alarming, the potential effect on long-term average recharge estimates is minimal, so the surface water use data provided by the TNRCC Water Master was used directly. Areal precipitation computation, streamflow naturalization, and potential runoff calculation were all accomplished using techniques described in referenced studies.^{1,2}

HDR, Op. Cit., September, 1993.

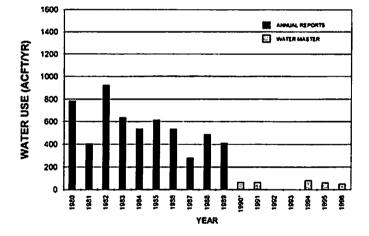
² HDR, Op. Cit., May, 1991.

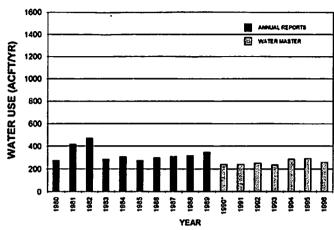


3

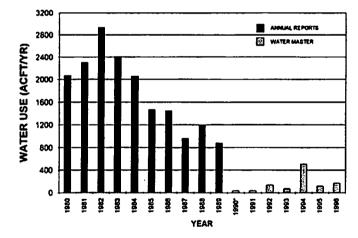


FRIO RIVER ABOVE CONCAN





NUECES RIVER ABOVE LAGUNA



* No records of use available from TNRCC Water Master's office.

Water use assumed equal to that reported in 1991.

EDWARDS AQUIFER **RECHARGE UPDATE**

TRANS TEXAS WATER PROGRAM / WEST CENTRAL STUDY AREA

REPORTED WATER USE COMPARISONS

HDR Engineering, Inc.

FIGURE 2.0-1

BLANCO RIVER ABOVE WIMBERLEY

3.0 RECHARGE SUMMARY AND COMPARISONS

Methodologies previously developed and applied by HDR in the computation of Edwards Aquifer recharge on a monthly timestep are described at length in studies prepared under the sponsorship of the Edwards Underground Water District¹ and the Nueces River Authority.² For consistency with these referenced studies, recharge estimates for the 1990-96 period have been computed using methodologies and assumptions identical to those previously applied. Resulting recharge estimates are summarized by major river basin in the following subsections and compared to those estimates prepared by the USGS. A comprehensive summary of historical Edwards Aquifer recharge estimates by river and recharge basin for the full 1934-96 historical period is included as Appendix A.

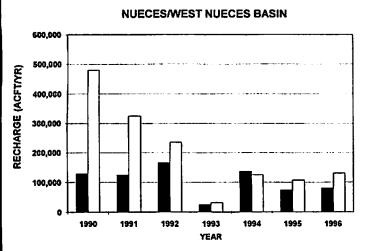
3.1 Nueces River Basin

The Nueces River Basin has been subdivided into four recharge basins identified in Figure 1.0-1 as the Nueces / West Nueces, Frio / Dry Frio, Sabinal, and the Area Between Sabinal and Medina Basin (which includes Seco, Hondo, and Verde Creek as well as several smaller tributary streams). In addition to naturally occurring recharge in the Nueces River Basin, the EAA (formerly EUWD) has constructed projects located on Seco, Parkers, and Verde Creek which serve to enhance recharge. Recharge associated with these projects was provided by the EAA for inclusion in the recharge basin summaries presented herein.

Figure 3.1-1 summarizes both HDR and USGS estimates of Edwards Aquifer recharge for each recharge basin within the Nueces River Basin for the 1990-96 historical period. Based on the full 1934-96 historical period, record high annual recharge volumes (432,412 acft) for the Sabinal River and the Seco, Hondo, and Verde Creek basins occurred in 1992 while a record low annual recharge volume of only 1,894 acft was computed for the Hondo Creek basin in 1996. It is readily apparently in Figure 3.1-1 that USGS recharge estimates in the wettest years are sometimes more than double those computed by HDR. There are several fundamental differences between certain recharge calculation procedures employed by the USGS and HDR,

¹ HDR, Op. Cit., September, 1993.

² HDR, Op. Cit., May, 1991.



600,000 600,000 RECHARGE (ACFT/VR) 400,000 300,000 200,000 100,000 ٥ 1990 1991 1992 1993 1994 1995 1996 YEAR

FRIO/DRY FRIO BASIN



1992

1993

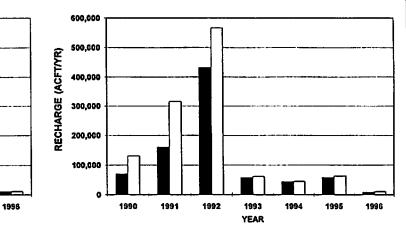
YEAR

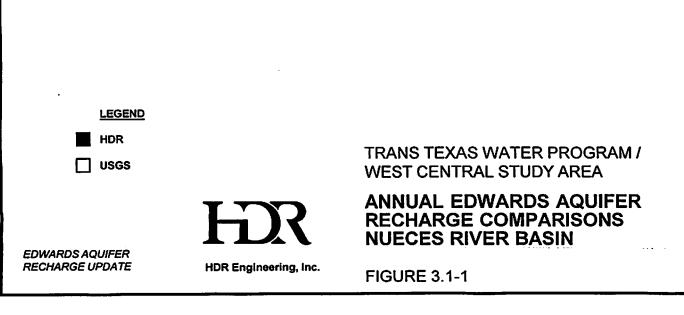
1994

1995

1991

AREA BETWEEN SABINAL AND MEDINA BASIN





600,000

500,000

400,000

300,000

200,000

100,000

0

1990

RECHARGE (ACFT/YR)

such as areal precipitation calculation, potential runoff estimation, and accounting for reported water rights diversions. The extreme difference in wet year estimates, however, is believed to be associated with the USGS application of "base flow curves" relating base flow upstream of the Edwards Aquifer outcrop to storage in the Edwards Plateau Aquifer contributing to base flow.³

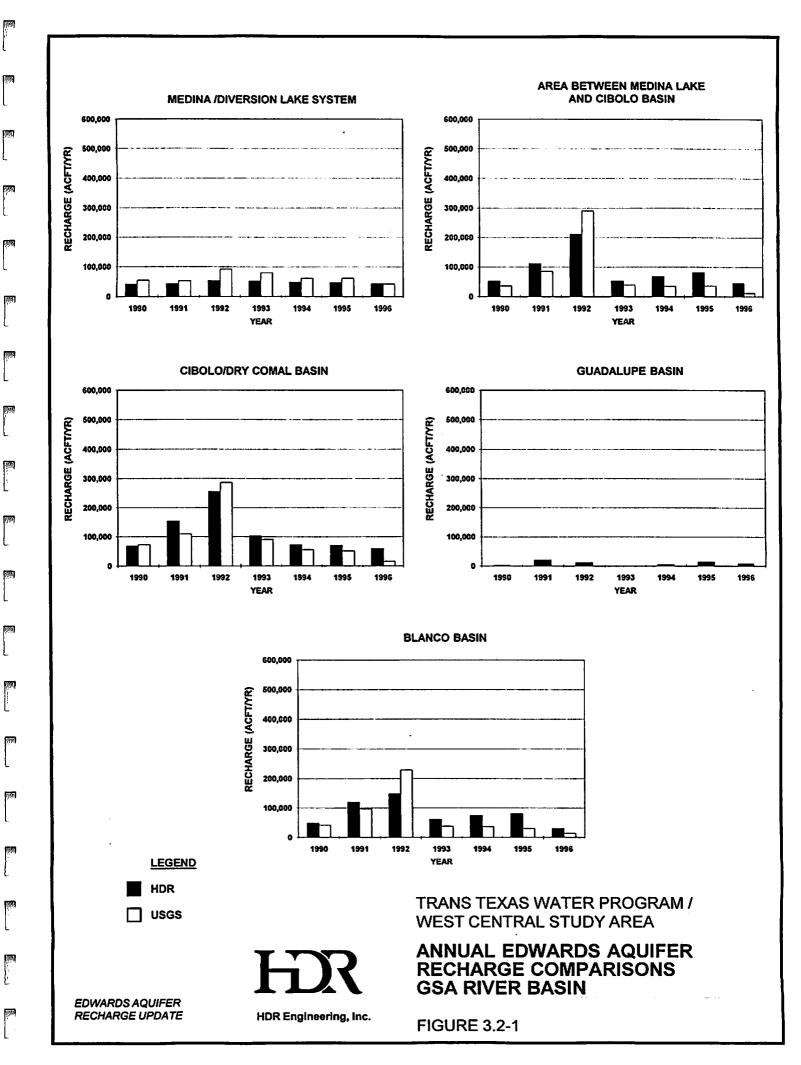
3.2 Guadalupe - San Antonio River Basin

The Guadalupe - San Antonio River Basin has been subdivided into five recharge basins identified in Figure 1.0-2 as the Medina River, Area Between Medina and Cibolo (which includes San Geronimo, Helotes, Leon, and Salado Creek as well as several smaller tributary streams), Cibolo and Dry Comal, Guadalupe, and Blanco. In addition to naturally occurring recharge in the Guadalupe - San Antonio River Basin, the EAA has constructed one recharge project located on San Geronimo Creek and the Natural Resources Conservation Service (formerly Soil Conservation Service) has constructed numerous Flood Retardation Structures (FRS) in the Salado, Dry Comal, and Upper San Marcos basins which serve to enhance recharge. Recharge associated with the San Geronimo project was provided by the EAA for inclusion in the recharge basin summaries presented herein. Estimates of historical recharge enhancement associated with the FRS were computed by HDR using methodologies summarized in a previous study.⁴

Figure 3.2-1 summarizes both HDR and USGS estimates of Edwards Aquifer recharge for each recharge basin within the Guadalupe - San Antonio River Basin for the 1990-96 historical period. Based on the full 1934-96 historical period, record high annual recharge amounts for the Upper San Marcos River, Salado Creek, and combined Cibolo and Dry Comal Creek basins occurred in 1992. With the exceptions of the Medina / Diversion Lake System and the Guadalupe Basin, it is apparent in Figure 3.2-1 that HDR recharge estimates generally exceed those prepared by the USGS. This is likely due to the selection of different partner areas for estimating potential runoff from the areas in which the Edwards formation outcrops. Again, the marked difference in Blanco River recharge estimates for 1992 (which was the wettest year

³ USGS, "Method of Estimating Natural Recharge to the Edwards Aquifer in the San Antonio Area, Texas," Water Resources Investigations 78-10, April, 1978.

⁴ HDR, Op. Cit., September, 1993.



during the 1990-96 period) is likely explained by the USGS application of a base flow curve in their computation procedure.

Both the USGS and HDR estimates of annual recharge in the Medina / Diversion Lake System were computed using curves relating reservoir storage (or water surface elevation) to recharge rate. Applicable curves, however, were obtained from different sources. The USGS uses curves originally derived by Lowry⁵ and HDR uses curves developed by Espey Huston & Associates.⁶ It is likely that both sets of curves will soon be superseded by information in an upcoming USGS report on the Medina Lake Project which is presently under internal review.⁷

Also of note in Figure 3.2-1 is that HDR reports small annual estimates of Edwards Aquifer recharge occurring in the intervening Guadalupe River watershed between Canyon Reservoir and New Braunfels. The USGS reports that "the Guadalupe River crosses the infiltration area of the Edwards Aquifer, but does not contribute recharge in significant quantities."⁸ HDR estimates indicate that annual recharge occurring in this area was as great as 20,363 acft during the 1990-96 period, but represents less than 2 percent of the long-term (1934-96) average recharge for the Edwards Aquifer in the Nueces and Guadalupe - San Antonio River Basins.

3.3 General Comparisons

As indicated in Appendix A, Edwards Aquifer recharge averaged about 652,700 acft/yr during the 1934-96 historical period. This is comparable to the published USGS estimate of 668,600 acft/yr which is about 2.4 percent greater. Table 3.3-1 and Figure 3.3-1 provide convenient summaries for geographical comparison of long-term average Edwards Aquifer recharge estimates developed by HDR and the USGS. Substantial differences, both in terms of volume and percentage, are readily apparent in specific recharge basins as only the Cibolo / Dry Comal recharge basin shows estimates within 10 percent of one another. In order to understand the differences between the HDR and USGS recharge estimates, basic methodologies and

⁵ Lowry, R.L., "Recharge to the Edwards Ground Water Reservoir," San Antonio City Water Board, 1955.

⁶ Espey, Huston & Associates, Inc., "Medina Lake Hydrology Study," Edwards Underground Water District, March, 1989.

⁷ Lambert, R., Personal Communication, USGS, December, 1997.

⁸ USGS, Op. Cit., April, 1978.

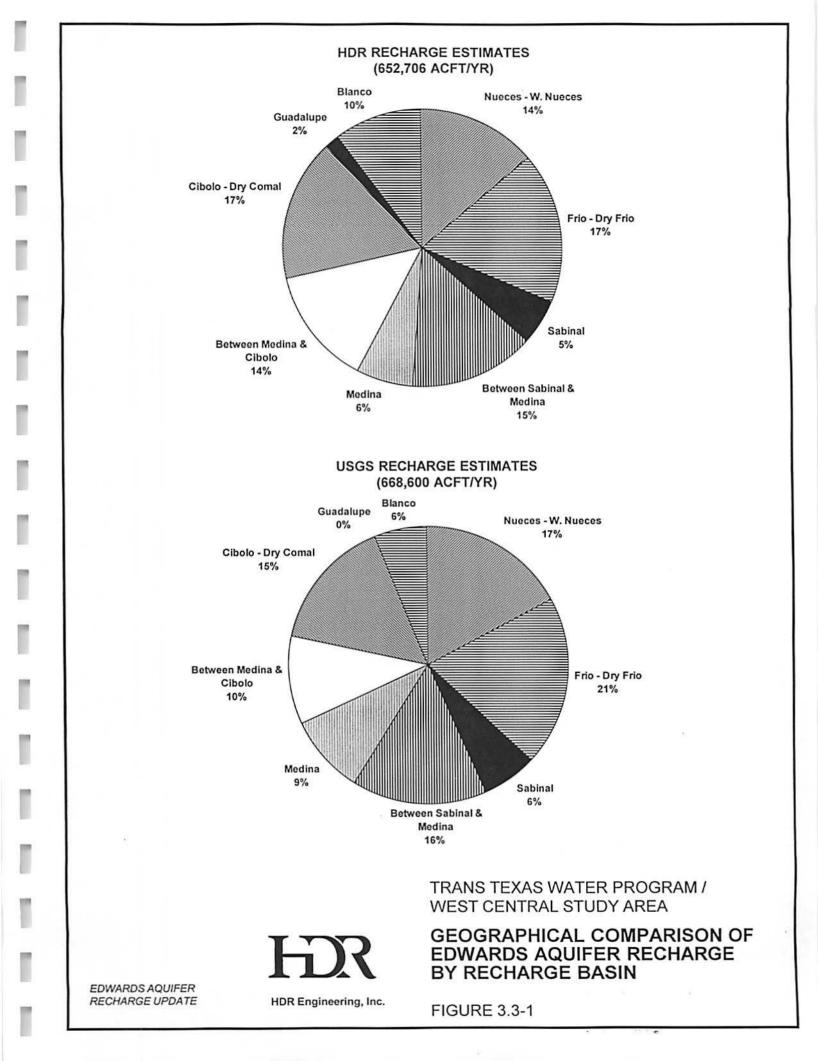
assumptions must be considered in some detail. The principal differences in recharge calculation methodology and procedures are associated with:

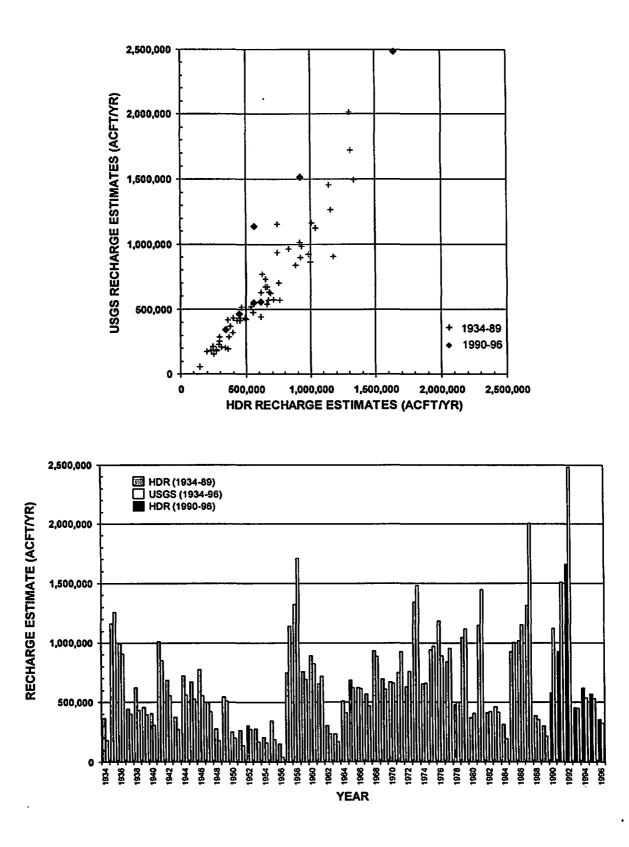
- Estimation of monthly potential runoff volumes for gaged and ungaged areas located atop the recharge zone (partner watershed, drainage area, areal precipitation, soil-cover complex, etc.);
- Base flow separation and accounting for storage in the Edwards Plateau Aquifer;
- Utilization of differing curves relating storage and recharge for the Medina / Diversion Lake System;
- Consideration of relatively small annual volumes of recharge for the Guadalupe River recharge basin; and
- Accounting for relatively small reported historical surface water diversions and treated effluent discharges.

For more detailed information on these differences, the reader is directed to referenced reports prepared by HDR and the USGS.

Table 3.3-1														
Summary of Average Historical Edwards Aquifer Recharge by Basin (1934-96)														
	HDR USGS													
	· · ·	Recharge	Recharge											
River		Estimate	Estimate	Difference	Percent									
Basin	Recharge Basin	(Acft/Yr)	(Acft/Yr)	(Acft/Yr)	Difference									
·· · · _	1. Nueces - W. Nueces	90,555	115,600	25,045	27.7%									
	2. Frio - Dry Frio	114,824	131,900	17,076	14.9%									
	3. Sabinal	33,201	41,400	8,199	24.7%									
	4. Between Sabinal & Medina	95,818	105,500	9,682	10.1%									
Nueces	SUBTOTAL	334,398	394,400	60,002	17.9%									
	5. Medina	42,393	61,000	18,607	43.9%									
	6. Between Medina & Cibolo	88,289	68,600	-19,689	-22.3%									
San	7. Cibolo - Dry Comal	110,307	103,300	-7,007	-6.4%									
Antonio	. SUBTOTAL	240,989	232,900	-8,089	-3.4%									
	8. Guadalupe	10,997	0	-10,997	-100.0%									
	9. Blanco	66,322	41,300	-25,022	-37.7%									
Guadalupe	SUBTOTAL	77,319	41,300	-36,019	-46.6%									
	TOTAL	652,706	668,600	15,894	2.4%									

Figure 3.3-2 provides two comparisons of HDR and USGS recharge estimates on a year by year basis for the entire 1934-96 historical period. Note that Edwards Aquifer recharge in 1992 was the greatest during the historical period (based on either HDR or USGS estimates) and





TRANS TEXAS WATER PROGRAM / WEST CENTRAL STUDY AREA

LONG TERM EDWARDS AQUIFER RECHARGE COMPARISONS

EDWARDS AQUIFER RECHARGE UPDATE

1

燃料

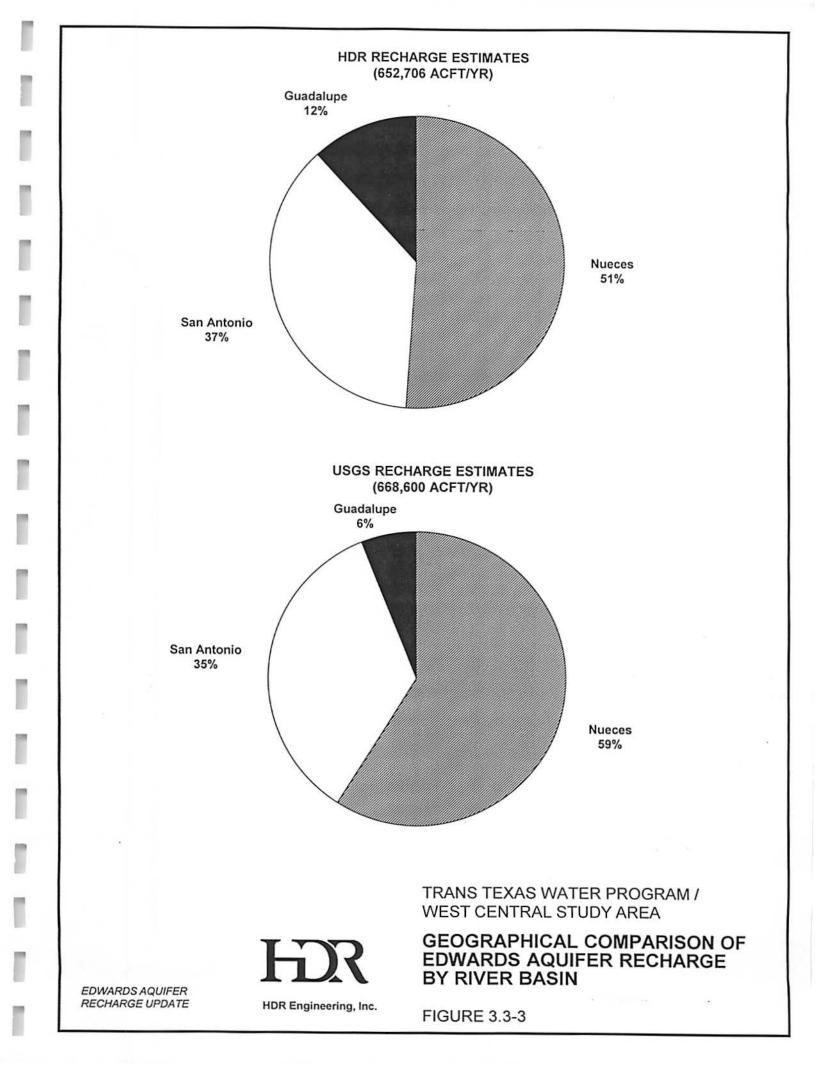
FIGURE 3.3-2

HDR Engineering, Inc.

exceeded the next highest year by almost 20 percent. As is apparent in this figure, USGS recharge estimates are substantially greater than HDR estimates in the wettest years and somewhat less than HDR estimates in the driest years.

A comparison of the geographical distribution of long-term average Edwards Aquifer recharge on a river basin scale is presented in Figure 3.3-3. Clearly, USGS estimates are greater in the Nueces River Basin and substantially less in the Guadalupe River Basin. This difference in geographical recharge distribution is quite significant with respect to both calibration and application of Edwards Aquifer models. For example, complete reliance on USGS recharge estimates could result in overestimation of aquifer storage in the western counties and underestimation of reductions in well levels in San Antonio and springflows in Comal and Hays County. Similarly, complete reliance on USGS recharge estimates could result in overestimation of the effects of aquifer-wide pumpage on San Marcos Springs discharge due to underestimation locally occurring recharge in Hays County. Preliminary comparisons⁹ indicate that the GWSIM4 model (originally calibrated using USGS recharge estimates) more accurately simulates historical springflows and Bexar County Monitoring Well levels when using HDR recharge estimates.

⁹ HDR, Letter to Rick Illgner (EUWD), February, 28, 1994.



4.0 RECOMMENDATIONS

The hydrologic extremes experienced during the 1990-96 historical period serve to reemphasize the importance of hydrologic data collection and periodic reassessment of methodologies applied in estimation of Edwards Aquifer recharge. The following are several recommendations regarding opportunities for improvement of recharge estimates:

- Data collection efforts implemented through the EAA precipitation and streamflow gaging network should be published on an annual basis as this data can contribute significantly to the accuracy of areal precipitation, potential runoff, and recharge estimates for all areas over the Edwards Aquifer recharge zone.
- Results of the Medina Lake Project when completed by BMA, BCA, and the USGS should be used to revise recharge relationships presently used for the Medina / Diversion Lake System.
- Results of a series of streamflow measurements on the Guadalupe River between Canyon Reservoir and New Braunfels conducted by the EAA, TWDB, and Guadalupe-Blanco River Authority should be analyzed and published, and recharge computation procedures revised accordingly.
- USGS records should be researched to determine if estimates of surface runoff for the portion of Upper San Marcos watershed above the springflow/streamflow gaging station located on the San Marcos River (#08170000) can be developed.
- Potential linkage of the EAA precipitation gaging network to advanced radar systems capable of measuring and recording the spatial distribution of precipitation intensity during storm events should be considered to improve estimates of areal precipitation.
- An improved, unified methodology for recharge calculation incorporating the best features of HDR and USGS procedures should be developed considering appropriate information from other studies and especially the EAA's ongoing data collection efforts.

Development of the best possible recharge computation procedures and, in turn, the best estimates of historical recharge are logical prerequisites for calibration and application of the most accurate aquifer model(s) possible. Ultimately, the best practicable Edwards Aquifer model must be developed to provide a sound technical basis for regulatory applications by both the EAA and TNRCC. Such a model will also prove invaluable in the technical evaluation of potential water supply plans involving conjunctive water supply management for the San Antonio region.



Trans-Texas Water Program West Central Study Area

關係

1097 ||-||-

108 || |}

)))))) | |

.

Edwards Aquifer Recharge Update

A-1

							i				
					BLANCO RIVER			GUADALUPE			
	NUECES RIVER	FRIO RIVER	SABINAL	SECO HONDO		PARTNER AREA	BLANCO RIVER	NUECES	SAN ANTONIO		
	AND WEST NUECES RIVER	AND DRY FRIO RIVER	RIVER	CREEK	CREEK RIVER	YORK, SINK & PURGATORY CREEK	AND PARTNER AREA	RIVER BASIN TOTAL	RIVER BASIN TOTAL	HDR	
	RECHARGE	RECHARGE	RECHARGE	RECHARGE	RECHARGERGE	RECHARGE	TOTAL RECHARGE	RECHARGE	RECHARGE	TOTAL RECHARGE	
YEAR	(ACFT)	(ACFT)	(ACFT)	(ACFT)	(ACFT) IT)	(ACFT)	(ACFT)	(ACFT)	(ACFT)	(ACFT)	
1934	32889	34733	9383	6433	91 29549	45199	74748	112757	254319	367076	
1935	132831	321509	70191	112557	861: 22676	25559	48234	821230	344168	1165418	
1936 1937	209504	168722	48431	68287	452 22058	29587	51645	591367	401951	993317	
1938	40180 65582	72612 65301	21505 17441	14152 11345	179 21386 179 30418	20845 49773	42231 80191	187839 201744	255091	442931	
1939	219904	70809	16369	11545	155,12887	3868	16755	349833	420922 110170	622666 460003	
1940	71156	66029	18404	11474	174 22401	21203	43604	205687	201891	400003	
1941	102464	143376	44657	44741	456 43520	89837	133357	440104	569599	1009703	
1942	79296	85483	26855	16652	254: 35015	51894	86909	262625	420630	683255	
1943	53958	45464	14284	8393	117 20022	15956	35978	150236	226943	377179	
1944	96031	103684	22108	17417	218 33596	59580	93176	281724	438546	720269	
1945 1946	58175 105067	96568	27181 22448	23698	243 29689 154 35009	45012 58761	74701 93770	259785	410340	670124	
1947	100072	78828 81214	22448	13634 13217	154 35009	39035	66757	262234 247002	508917 255390	771151	
1948	55926	50832	12338	5392	33 16822	10592	27414	143631	135912	502392 279543	
1949	116471	111923	28351	23123	251. 18319	12904	31223	340768	207032	547800	
1950	59750	40605	14007	7994	100 15640	6853	24493	144837	107658	252706	
1951	57189	35366	6326	6016	73: 16051	19671	35722	138366	122515	260881	
1952	30359	27428	9703	5547	80 17489	20863	38352	95686		301615	
1953 1954	28556	30446	4619	7331	128 25585	31226	56811	99831	181026	260657	
1955	43278	27478	4017	4110	43 14676 36 15159	12185 17475	26862 32634	85996		206449	
1956	205474 25319	30774 9345	3206 4224	1544 1451	36 15159 56 12224	10106	22330	251170 58716		345996	
1857	104250	92879	22490	36832	395 45215	47132	92347	345221	402894	153383 748115	
1958	199766	255735	70117	90232	681 29650	56265	85915	754190		1325455	
1959	104504	172540	51863	30689	305 37040	52205	89245	431239		758720	
1960	95579	133568	60338	36273	411 50778	58381	109159	408007	481750	889756	
1961	123931	163843	52613	33361	352 12750	11017	23767	431577		655370	
1962 1963	57671	53458	5202	3378	37 19538 35 17194	20465	40003	130174		305702	
1964	47126 134656	38198 67406	6559	2060	23 aug	17336 19453	34530 40866	99173		239047	
1965	114710	90686	19902 44792	24441 26648	193 21413 323 33615	48391	82006	297884 346606		507960	
1966	123092	100837	33251	27216	257 33285	42767	76052	334719		686163 621900	
1967	82245	139032	39003	31879	197 17911	15798	33709	336115		564892	
1968	95065	163468	75500	78061	628 22694	37478	60172	547428		932495	
1969	120252	116967	35794	23247	342 27589	36011	65600	370617	323171	693788	
1970 1971	77417	124183	33424	16833	318 15243	45530	60772	312215		670119	
1972	167028 62963	178302	32839	43109	231 04770	19523 26028	36263 50807	502413		748175	
1973	146650	126817 210451	44298 56717	52925 91223	303 43999	84078	127966	362621 683221	268664	631284	
1974	45291	142177	41640	23890	894 43666 315 28982	46191	75173	312607	342715	1346006 655321	
1975	68271	127406	43110	40778	651 53952	86324	140276	391157	551648	942805	
1976	123277	250626	65417	50701	684 40947	63168	104115	623298	562925	1186223	
1977	18157	180811	60106	36184	553 37082	54249	91331	386873		839283	
1978 1979	63320	80599	37764	22716	237 23075	17529	40604	259370		476902	
1979	87809 52312	152844	52182	53886	/33 21480	<u>65294</u> 17504	105781	481572		1048508	
1681	52312 99236	68291 236963	23481	5980	114 30114	92668	36993 131762	173833		369935	
1982	40941	100673	79443	77455 13628	/32 21392	16383	37765	631763 202398		1150689	
1983	91758	80656	26657	13628	121 27578	33379	60957	250401	210570	412969	
1984	55405	46221	16221	11499	22407	13812	36219	144330		465829 316655	
1985	91366	172152	55982	58714	611 35/14	· 71924	107638	494970	431495		
1986 1987	96000	134742	46738	59598	AAE 33393	58844	92237	422239	595873		
1987	91216	288401	77781	109760	854 26619 25282	76824	105443	752261			
1989	52841 45222	97972	16541	4069	⁵¹ 26274	22883 23466	48165	186366		387751	
1990	129509	49915	8282	3330	25306	23466	49740	117262		297537	
1991	125575	153580	35504 46263	32768	241 35618	83581	46170	364276		574882	
1992	165437	296510	85215	63194 168238	50(43263	105703	148965	484905 979574			
1993	24385	75620	28933	24016	1364 27525 176 26649	34173	61698	185842		1661168	
1994	135924	144347	26385	10237	4.41 00040	38902	75550	349562	270700	454948 620262	
1995 1996	73991	106342	33694	21556	~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~	49373	80987	274629	293550	568180	
AVG	60489	68299	11107	4173	10003	13534		168775	185169	353944	
MIN	90555	114824	33201	31615	317 12224	38664	66322	334398	318308	652706	
MAX	18157 219904	9345 321509	3206	1451	16 53053	3868 105703	16092	58716	94666	153383	
		321309	85215	168236	1362	109703	159655	979574	681594	1661168	

•

-

•

HISTORICAL EDWARDS ACIUIFER RECHARGE ESTIMATES

YEAR 1934	NUECES RWER AND WEST NUECES RIVER RECHARGE (ACFT)	FRIO RIVER AND DRY FRIO RIVER RECHARGE (ACFT)	SABINAL RIVER RECHARGE (ACFT)	SECO CREEK RECHARGE (ACFT)	HONDO CREEK RECHARGE (ACFT)	VERDE & ELM CREEK RECHARGE (ACFT)	AREA BETWEEN SABINAL RIVER AND MEDINA RIVER RECHARGE (ACFT)	MEDINA LAKE AND DIVERSION LAKE RECHARGE (ACFT)	(ACFT)	AREA BETWEEN MEDINA LAKE AND SALADO CREEK ABOVE RECHARGE ZONE (ACFT)	SAN GERONIMO DAM RECHARGE (ACFT)	S. KADO CREEK RECHARGE (ACFT)	TOTAL RECHARGE (ACFT)	(ACFT)	DRY COMAL CREEK RECHARGE (ACFT)	CIBOLO CREEK AND DRY COMAL CREEK TOTAL RECHARGE (ACFT)	ESTIMAJED GUADALUPE RIVER RECHARGE (ACFT)	BLANCO RIVER RECHARGE (ACFT)	BLANCO RIVER PARTNER AREA YORK, SINK & PURGATORY CREEK RECHARGE (ACFT)	BLANCO RIVER AND PARTNER AREA TOTAL RECHARGE (ACFT)	NUECES RIVER BASIN TOTAL RECHARGE (ACFT)	GUADALUPE SAN ANTONIO RIVER BASIN TOTAL RECHARGE (ACFT)	HDR TOTAL RECHARGE (ACFT)
1935 1935 1936	32889 132831 209504	34733 321509 168722	9383 70191 48431	6433 112557 68287	9163 86159 45204	20156 97582	35752 296698	44497 47117	6388 32974	686/ 31360		33859 59167 69011	47114 123501	16848 84493	38790 34374		32323 6469	29549 22676	45199 25559	74748 48234	112757 821230	254319 344168	367076 1165418
1937 1938	40180	72612 65301	21505 17441	14152 11345	45294 17962 17908	51129 21428 24168	164710 53542 53420	53275 52235 48820	15614	30885 15384 26193	0	38400	131681 69398 116923		44154 32606 78144	78969	12258	21386	29587 20845	51645 42231	591367 187839	401951 255091	993317 442931
1939	219904	70609	16369	11545 11474	15549	15657 21206	42751	40604	4226	4663	0	9328	18217	11356		15327	31032 19266 26689	12887	49773	16755	201744 349833	420922	460003
1941 1942	102464 79296	143376 85483	44657 26855	44741 16652	45640 25497		149607 70991	43076	45120		. 0	83039 63483	171023	102432	98172 74746	200604	20000	43520	21203 89837 51894	133357	205687 440104 262625	201891 569599 420630	407578 1009703 683255
1943 1944	53958 96031	45464 103684	14284	8393 17417	11707 21856	16431 20627	36531 59900	42925	12957	13066	i 0	35990	62013	38514	35492 64032	74006		20022	15956	35978	150236 281724	420530 226943 438546	
1945 1946	58175 105067	96568 78828	27181 22448	23698 13634	24346 15454	29815 26802	77660 55891	42124 39602	33414		0				71717 94639	158219	16971	29689	45012		259785	436546 410340 508917	
1947 1948	100972 55926	81214 50832	19759 12338	13217 5392	15794 3399	16045 15744	45056 24535	38385 18890	11061	11101	0	24219 15382	46381	48102	39040 13704	87142	16725	5 27722	39035	66757	247002	255390	
1949 1950	116471 59750	111923 40605	28351	23123 7994	25142	35757	84022	30907 13274	15862			33401	64979	37414	31540	68954		18319	12904	31223	340768	207032	
1951 1952	57189 30359	35366 27428	6326 9703	8016 5547	7324 8050		39465 28197	10256	9726	10097	' Ö	9778 17230	29601	16856	13446	30302	16634 9841	4 16051	19671 20863	35722	138366 95686	122515 205928	260881 301615
1953 1954	28556 43278	30446 27478	4619 4017	7331 4110	12891 4374	15968 2739	36210 11223	19343 23430	5536	5902	· آ	17215 18500	28652	25078	33314		17827 18376	7 25585	31228 12185	56811	99831 85996	181026 120453	280857 206449
1955 1956	205474 25319	30774 9345	3206 4224	1544 1451	3627 5692		11715 19828	10789 11076		1145	i 0	6783 12244		2984		15177	27389	9 15159		32634	251170 58716		
1957 1958	104250 199768	92879 255735	22490 70117	36832 90232	39567 68194		125602 228572	29316 51508		30416 49554			121242				7977 6626		47132 56265		345221 754190	402894 571265	1325455
1959 1960 1961	104504 95579 123931	172540 133568	51863 60338	30689	30588 41160	41055 41089	102332 118522	<u>5293</u> 52554	36751	34909	0	24574	76193	121568	59948	181516	7648	9 50778	58381	109159	431239 408007	481750	758720 889756
1962 1963	57671 47126	163843 53458 38198	52613 5202 6559	33361 3378	35205 3794	22624 6671	91190 13843	48709	6924	19648 7265	i 0	19753 22656	36845	15857	21759	37616	5734 12355	5 19538	20465	40003	431577 130174		655370 305702
1964 1965	134656	67406 90686	19902	2060 24441 26648	2543 19384	32095	7290 75920		16588	16659) 0	22200	55448	46856	26452	73308	5107	7 21413	19453	40866	297884	210076	507960
1966 1967	123092 82245	100637 139032	33251 39003	27216	32312 25714 19708		98618 77539 75835	40895	14853	22851 14601		46493 48408	77862		54978	87608	4470		42767		334719	287181	
1968 1969	95065 120252	183488 116967	75500	78061 23247	62831 34233	52483 40125	73635 193375 97605	37456 48319 48448	28920	13891 29244		37913 50765 37075	108929	114126	55085	169211	430	6 22694	37478	60172	547428		032495
1970 1971	77417 167028	124183 178302	33424 32839	16833 43109	31828 29702	28530	77191	49115	25941	<u>31883</u> 32348 18352	0	34852	92941	92674	53411	146085	899	1 15243	45530	60772	312215	357904	670119
1972 1973	62963 146650	126817 210451	44298 56717	52925 91223	38546 89441	37071 88739	128543 269403	52951	12102	22671	i 0	27631	62404	67772	34090	101862	64	0 24779	2602	50807	362621		631284
1974 1975	45291 68271	142177 127406	41640 43110	23890 40778	31591 65172	26018	83499 152370	53005 53026	13937	24472	2 0	30466	6887	5 80363	53837	134200	1148	1 28982	4619	75173		342715	5 655321
1976 1977	123277 18157	250626 180811	65417 60106	50701 36184	68496 55301	64781 36313	183978 127799	52628 52908	40204	36837	7 Ö	102385	5 17942	8 123955	9352	217484			6316	3 104115	623296	56292	639283
1978 1979	63320 87809	80599 152844	37764 52182	22716 53886	23751	31220 61484	77687 188737	51880 53015	10422	17181	i Ō		5263	B 31904	35328	67232		8 23075	1752	9 40604	259370 481572	217532	6 1048508
1980 1981 1982	52312 99236	68291 236963	23481 79443	5580 77455	11429 73218	12339 65448	29749 216121	50168 52361	3 7619 40474					31629 1 112245			307		9266	8 131782	631763	516927	7 1150689
1983 1984	40941 91758 55405	100673 80656 46221	22684 26657	13628 8909	12178 17222		38100 51330	48329			91	30968	4023	5 32516	2975	62270	363	7 27578	3337	9 60957	25040	21542	8 465828
1985 1986	91366	40221 172152 134742	16221 55982	11499 58714	8109 61388	6875 55369	26483 175470	44162	21945		-		4 13420		7214	144796	69	1 35714	· 7192	4 107638	49497	43149	5 926464
1987 1988	91216 52841	288401 97972	46738 77781 16541	59598 109760	44939 85586	40222 99518 6020	144759 294863	53217	41404	61814 66330	963 0 1176	7750	9 18641	9 122444	8648	5 208930	836	8 26619) 7682	4 105443	3 75226	59587 56237	7 1314638
1989 1990	45222	49915	8282	4069 3330 32768	8924 5205	5308	19012 13843	50855 44998	4653	7242		30243	9 4195	4 13457	2349	36948	663	18 25282 15 26274	1 2346	6 49740	11726	2 16027	6 297537
1991 1992	125575	153560 296510	46263 85215	63194 168236	24530 50079 136235	12131 46214	69429 159487	41708 42601	27396	18926	i 1647	6253	5 11050	4 52983	9967	5 152656	2036	3 35618	8358	1 1191 9 5	3 48490	5 44532	6 930230
1993 1994	24385 135924	75620 144347	28933 26385	24016	130235 17807 14851	127942 15082 17818	432412 56905	53197 51838	6266	26276	334	1972	8 5260	4 7063	7. 3232	9 102966	3	0 27525	5 3417	3 61698	8 18584		8 454948 0 620262
1995 1996	73991 80489	108342 68299	33694	21556 4173	14651 20052 1894	17818 16993 2814	42906 58602 8881	48875 47498	11721	17365 17324	i 0 51	4007	7 8009	3 27324	4332	4 7064	3 1432	07 36640 25 31613 33 16689	3 4937	3 80987	7 27462	9 29355	0 568160 9 353944
AVG MIN	90555	<u>114824</u> 9345	33201 3206	31615 1451	31212		95818	42397	21044	22598	151	1152	6 8828	9 63419	4688	8 11030	7 1099	2745	3886	4 6632	2 33439	8 31830	24 032/001
MAX	219904	321509	85215	168236	136235		7290 432412	10256 53275	910 57142					9 1683 8 149136	397 12114	1 1517 6 25548		0 12224 70 53952			5 97957		6 153383 94 1661168

-

Aquifer Simulations With Alternative Recharge Estimates

-

T

I

-

-

[

T

T

-

T

HR

February 28, 1994

Mr. Rick Illgner General Manager Edwards Underground Water District 1615 N. St. Mary's San Antonio, Texas 78212

Dear Mr. Illgner:

Pursuant to various discussions with you and members of your staff, we have enclosed a series of graphs which compare the springflows and aquifer levels from the Texas Water Development Board Edwards Aquifer model to historic observations using the USGS recharge estimates and the HDR recharge estimates. The plots compare the results of the TWDB model at Comal Springs, San Marcos Springs, and at the Bexar County Monitoring Well (J-17). This data was provided to us by the TWDB during our continuing discussions with the USGS and the TWDB regarding technical issues related to Edwards Aquifer recharge. It is important to note that the TWDB model has only been calibrated to the USGS recharge estimates. The calibration was performed with the emphasis on matching the drought conditions using the USGS recharge estimates. HDR recharge estimates were input into the model in place of the USGS recharge estimates, however, no recalibration of the model was performed when the HDR recharge estimates were simulated.

Figure 1, Figure 2, and Figure 3 show time traces of Comal Springs discharge, San Marcos Springs discharge, and Bexar County Monitoring Well (J-17) level, respectively. These three plots show the simulated records from the TWDB model using the USGS and HDR recharge estimates in comparison with historical observations for the drought period (1947-59) and for a more recent period (1978-89). Figure 4, Figure 5, and Figure 6 present a comparison of the simulated records using the USGS recharge estimates and HDR recharge estimates versus the historical observations at the three locations. If the results of the simulations exactly matched the historical observations, the data points would fall on the line shown on the individual graphs. Some general comments on each of figures are as follows:

• Figure 1 - Comal Springs Time Trace

Figure 1 shows the time traces of simulated and observed springflow for the 1947-59 period and 1978-89 period. In general, the HDR recharge estimates provided a closer approximation of historical springflows during the 1947-59 period than did the USGS recharge estimates. Using the HDR recharge estimates, the model showed that

HDR Engineering, Inc.

Suite 400 3000 South IH 35 Austin, Texas 78704-6536 Telephone 512 442-8501 Mr. Rick Illgner February 28, 1994 Page 2

> Comal Springs did not cease to flow in 1956 as occurred historically. This may be due to the fact that the TWDB model was not recalibrated using the HDR recharge estimates. A closer approximation of historical springflow using the HDR estimates was especially evident for the wet period following 1956. For the more recent period of 1978-89, the HDR recharge estimates provided a more accurate simulation of historical springflows for the higher flow periods, however when historical flows were in the range of 200 cfs to 300 cfs, the USGS recharge estimates appeared to produce improved results.

• Figure 2 - San Marcos Springs Time Trace

Figure 2 shows the time traces of simulated and observed springflow for the 1947-59 period and 1978-89 period. San Marcos Springs showed the most variability in the comparisons of historical to simulated springflows using the HDR recharge estimates and USGS recharge estimates. For both periods, the HDR recharge estimates simulated historical springflows more accurately than did the USGS recharge estimates. The TWDB model tends to support the belief that springflow at San Marcos Springs is heavily influenced by the recharge that occurs locally (i.e. Blanco River, Upper San Marcos River, Guadalupe River).

• Figure 3 - Bexar County Monitoring Well (J-17) Time Trace

Figure 3 shows the time traces for the 1947-59 period and 1978-89 period for the well level at the Bexar County Monitoring Well J-17. The TWDB model results showed that HDR recharge estimates simulated historical levels more accurately than did the USGS recharge estimates during the depths of the drought (1952-56) and performed better overall for the 1947-59 period. For the 1978-89 period, both sets of recharge estimates produced simulated levels that are lower than the historical levels, although the USGS recharge estimates did tend to produce slightly better results during this period than the HDR recharge estimates.

• Figure 4 - Comal Springs

Figure 4 shows the historical flows compared to the simulated flows obtained using the HDR and USGS recharge estimates for the periods of 1947-59 and 1978-89 for Comal Springs. As shown in the Figure 1 time trace, the HDR recharge estimates

Mr. Rick Illgner February 28, 1994 Page 3

provided a better match throughout the range of flows, although simulated springflows did not cease in 1956. For the same period, the USGS recharge estimates were consistently lower than historical conditions, except for occasionally high values. For the 1978-89 period, the HDR recharge estimates show a questionable fit in the 200 cfs to 300 cfs range. For the 1978-89 period, the USGS recharge estimates show a somewhat better fit when springflow is below 300 cfs. When historical flows exceeded 300 cfs, the USGS recharge estimates resulted in simulated flows that were significantly higher than historical flows. The HDR recharge estimates tended to provide better results for flow conditions above 300 cfs, although simulated flows were slightly higher than historical flows.

• Figure 5 - San Marcos Springs

Figure 5 shows the historical flows compared to the simulated flows obtained using HDR and USGS recharge estimates for the periods of 1947-59 and 1978-89 for San Marcos Springs. For the 1947-59 period, the HDR recharge estimates produced a better long-term volume match to historical flows than did those produced using the USGS estimates. The HDR recharge estimates did produce more variation in simulated flows than the USGS recharge estimates. Both sets of recharge estimates produced simulated flows which indicate a questionable calibration of the TWDB model. For the higher flow conditions, both sets of recharge estimates tended to simulate springflows which were less than historical flows. Similar observations are noted for the 1978-89 period.

• Figure 6 - Bexar County Monitoring Well (J-17)

Figure 6 shows the historical well levels compared to the simulated levels obtained using the HDR and USGS recharge estimates for the periods of 1947-59 and 1978-89 for the Bexar County Monitoring Well (J-17). For the 1947-59 period, the HDR recharge estimates provided results which more accurately simulated historical levels than did those produced using the USGS recharge estimates. The HDR recharge estimates tended to closely match historical levels below 650 ft-msl. However, for higher well levels, the levels calculated by the TWDB model using the HDR estimates tended to be lower than historical levels. For the 1947-59 period, the USGS recharge estimates produced levels which were consistently lower than historical levels throughout the range of well levels. For the 1978-89 period, both sets of recharge estimates produced levels which were consistently lower than

Mr. Rick Illgner February 28, 1994 Page 4

historical levels, although the USGS recharge estimates were slightly better. The results for the 1947-59 period and 1978-89 period raise questions as to the adequate calibration of the TWDB model for aquifer levels above 650 ft-msl. As stated previously, the TWDB model was calibrated to most accurately simulate low aquifer levels and springflows.

This letter is provided with the intent of providing a brief, general assessment of the ability of the TWDB Edwards Aquifer model to simulate key springflow and well levels using alternative recharge estimates developed by HDR and the USGS Overall, the TWDB Edwards Aquifer model seemed to more accurately simulate historical observations using the HDR recharge estimates. The TWDB model was calibrated to drought conditions, when the aquifer levels and springflows were low, using the USGS recharge estimates. It is possible that, if the model were recalibrated using the HDR recharge estimates, more accurate results could be obtained. Both sets of recharge estimates suggest that the TWDB model needs to be better calibrated in the San Marcos Springs area and for mid-range to higher aquifer level conditions.

If you have any questions or comments, please contact any of us at your convenience.

Sincerely,

HDR Engineering, Inc.

K. Vanh

Kenneth L. Choffel, P.E. Vice President

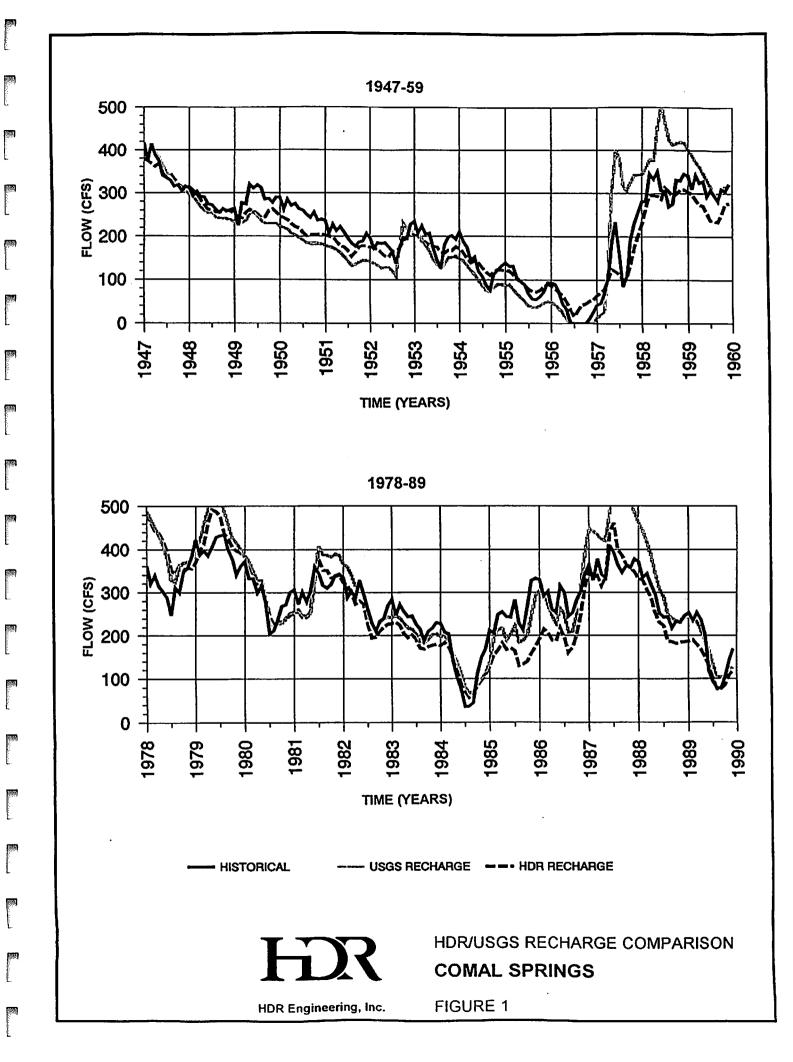
Kelly J. Kaatz, P.E. Project Engineer

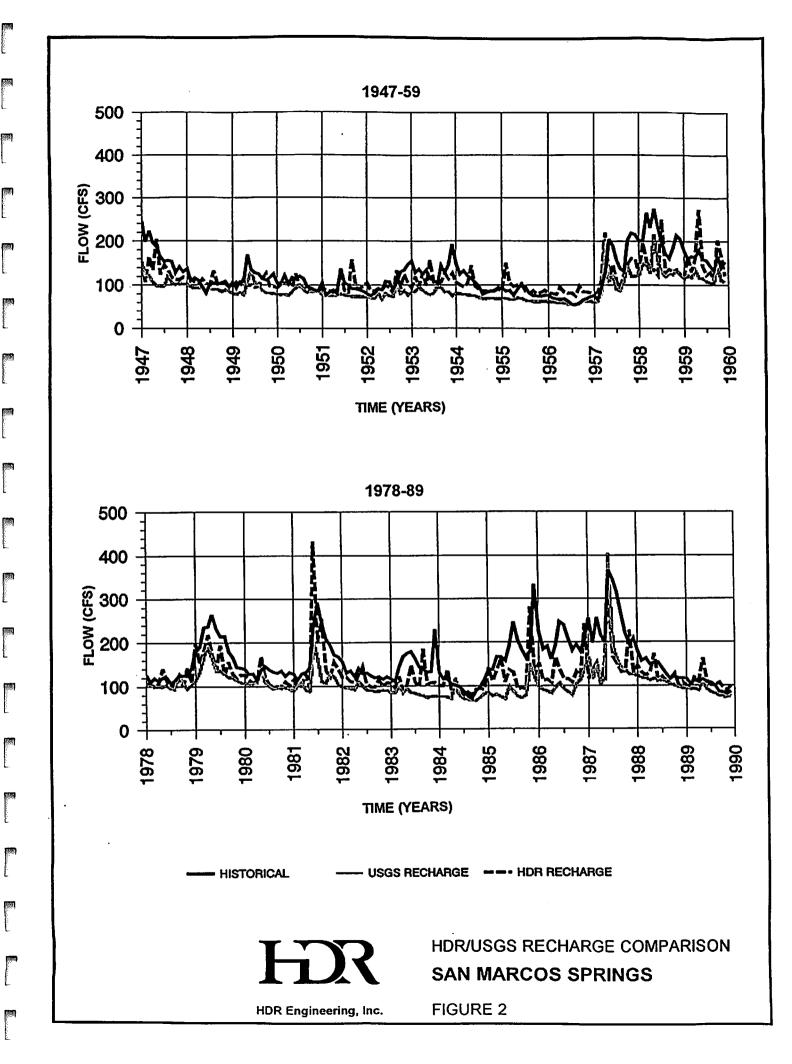
cc: Steve Walthour, EUWD Greg Rothe, G.E. Rothe Co.

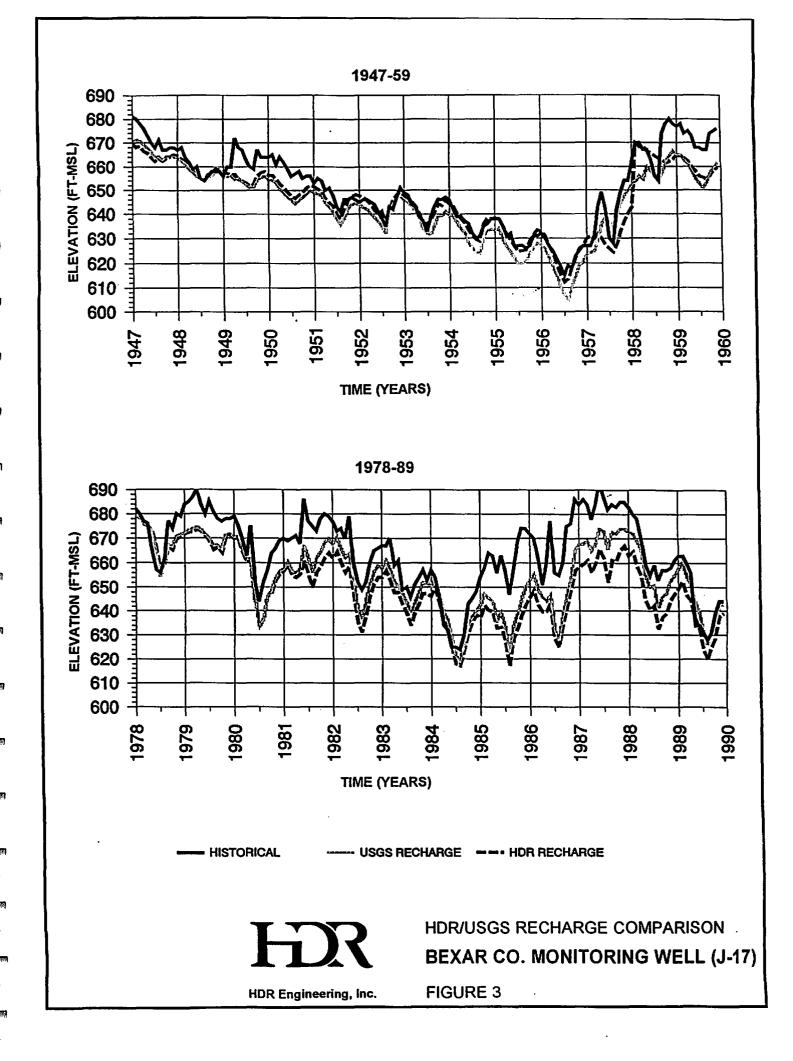
W:\KAATZ\WALTHOUR.LTR

1 L. Vanh

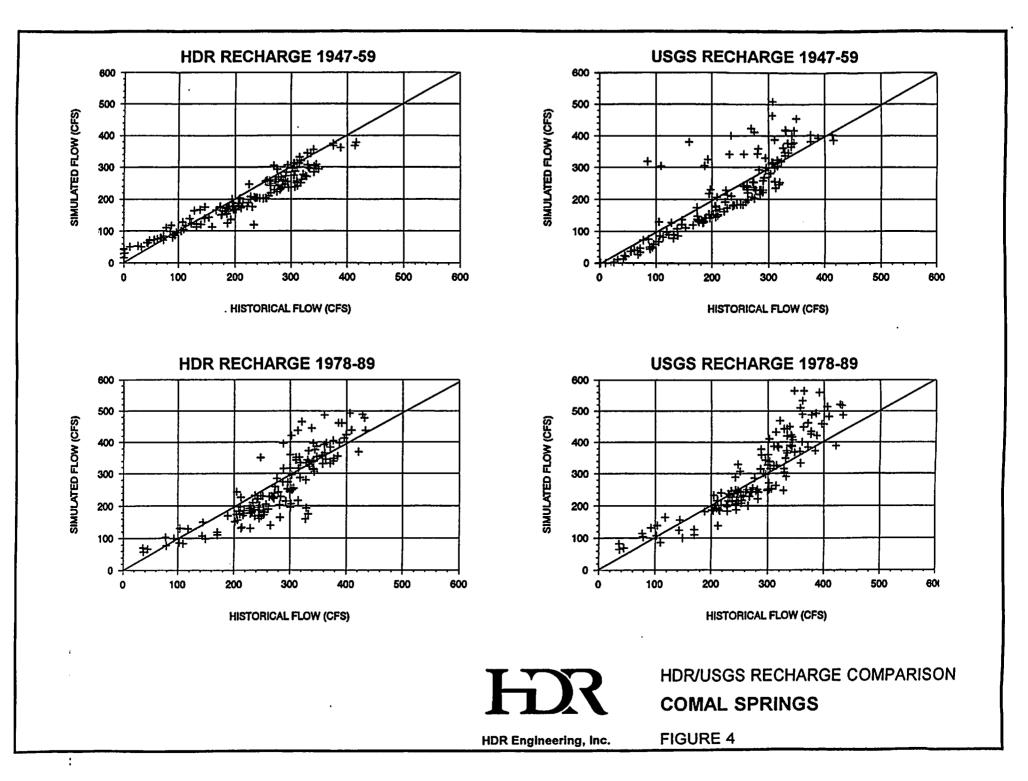
Samuel K. Vaugh, P.E. Project Manager



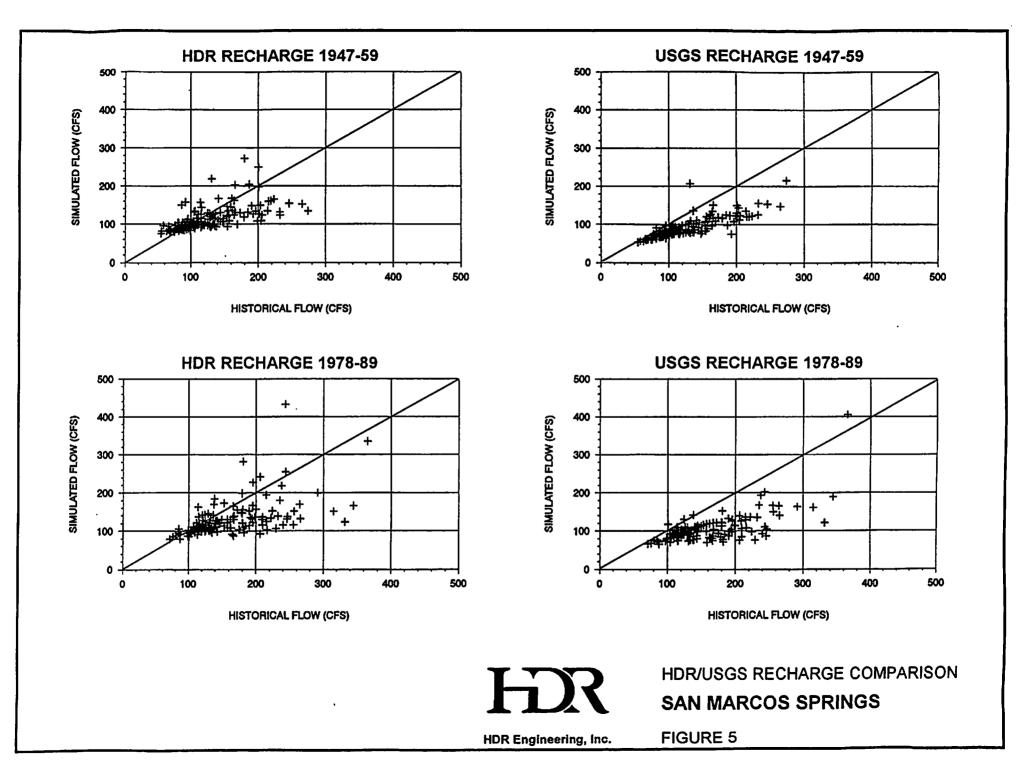


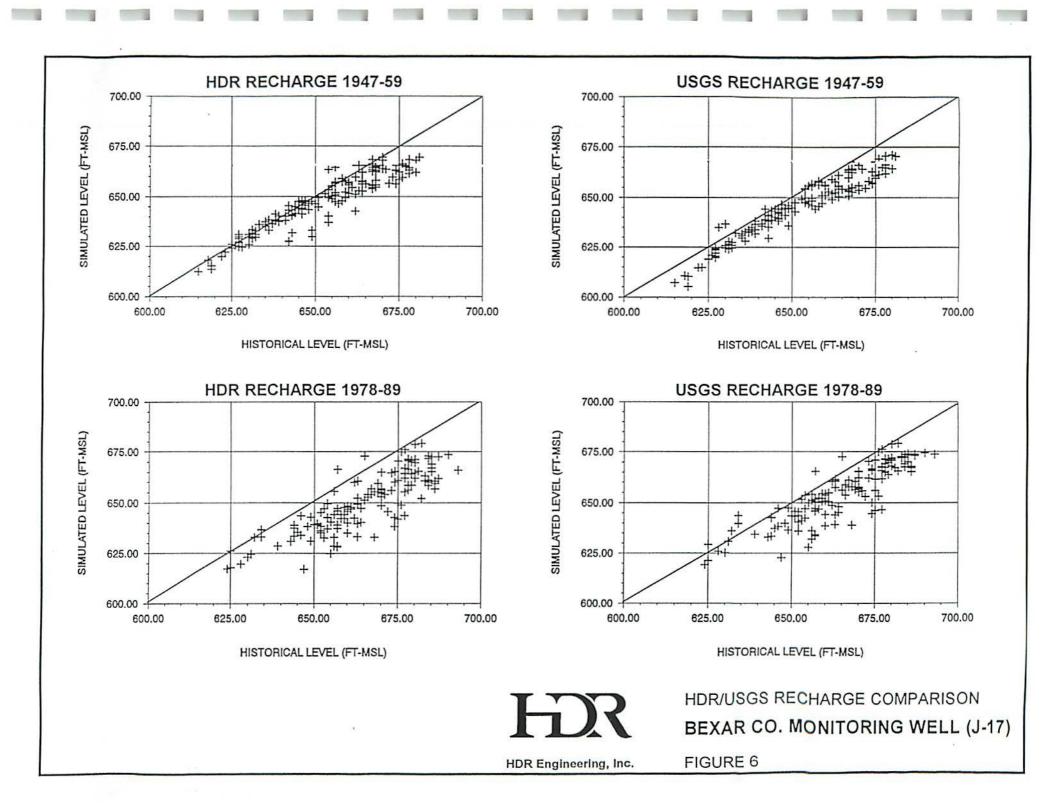


圞









Some Key Issues For Improved Edwards Aquifer Modeling

I

[

-

[

T

1

-

l

-

Selected Key Issues and Short-Term Objectives for Recalibration of the TWDB Edwards Aquifer Model (GWSIM4)

HDR Engineering, Inc. and LBG-Guyton Associates February 14, 1995

Following is a brief summary list of selected key issues and short-term objectives for recalibration and improvement of the Texas Water Development Board (TWDB) Edwards Aquifer Model (GWSIM4). Items comprising this list are either based on our own observations and experience or on our understanding of the observations and experience of others actively involved with the development and application of Edwards Aquifer models. It is recognized that simulation of the physical processes occurring in the Edwards Aquifer is an evolving science, hence both short-term and long-term objectives should be considered for the collection of basic data and development of model capabilities. The following list focuses on relatively short-term objectives which we believe can be reasonably achieved using data presently available and the existing model format.

Model Recalibration

- 1. Modify recharge estimates to reflect results of recent aquifer divide study which indicates that recharge in the Onion Creek watershed contributes to the San Antonio portion of the Edwards Aquifer. This study was conducted by LBG-Guyton for the Edwards Underground Water District (EUWD) and is presently in draft form pending approval by EUWD staff and Board of Directors. Consideration of Onion Creek recharge could help resolve the fact that the GWSIM4 model significantly underestimates discharge from San Marcos Springs.
- Estimates of historical recharge of the Edwards Aquifer have been developed 2. for the 1934-89 period by both the U.S. Geological Survey (USGS) and by HDR in the course of studies performed for the EUWD and others. Although GWSIM4 was calibrated using USGS estimates of historical recharge, the TWDB has performed numerous simulations with GWSIM4 using both USGS and HDR/EUWD recharge estimates during the past several years. In late 1993, comparisons of GWSIM4 results using both sets of recharge estimates were made. When these results were compared to actual historical springflows and well levels, it was found that results based on HDR/EUWD recharge estimates more closely approximated observed values (See Table 1). This finding is significant in that, even without recalibration of the model, the HDR/EUWD recharge estimates produced more accurate results. Based on these comparisons and other considerations summarized in the following paragraph, all GWSIM4 simulations used in the Trans-Texas Water Program were performed using HDR/EUWD estimates of historical recharge. Therefore, we strongly recommend that HDR/EUWD estimates of historical Edwards Aquifer recharge (as modified in Item 1) be adopted for recalibration of GWSIM4.

Table 1Comparison of GWSIM4 ResultsUsing HDR/EUWD and USGS Recharge EstimatesTo Historical Springflows and Well Levels ¹									
	Recharge Estimates with GWSIM4 Results Most Closely Approximating Historical Springflow or Well Level ²								
Location	High Range	Middle Range	Low Range	Lowest Range					
Comal Springs	HDR/EUWD	HDR/EUWD	HDR/EUWD	USGS					
San Marcos Springs	HDR/EUWD	HDR/EUWD	HDR/EUWD	USGS					
J-17	HDR/EUWD	HDR/EUWD	HDR/EUWD	HDR/EUWD					

HDR/EUWD recharge estimates use updated drainage areas in the Nueces River Basin, account for historical diversions, account for differences in soil cover complex between areas upstream of and directly over the outcrop, and use improved estimates of areal precipitation. HDR/EUWD estimates of historical recharge account for recharge in the Guadalupe River Basin above New Braufels and are significantly greater than U.S. Geological Survey (USGS) estimates in the Upper San Marcos River watershed. Application of the GWSIM4 model using HDR/EUWD recharge significantly improves simulation of discharge from San Marcos Springs. Although HDR/EUWD recharge estimates are by no means perfect, we feel that they represent significant improvement over USGS estimates (especially in the eastern portions of the recharge zone) and should be used until additional information becomes available from the EUWD gaging network, USGS/BMA Medina Lake studies, USGS/GBRA Guadalupe River studies, etc.

Consider calibration to a variety of monitoring well levels and springs, rather than focusing on the period of zero flow at Comal Springs. Although it correctly simulates the duration of flow cessation at Comal Springs, the existing GWSIM4 model underestimates both Comal Springs discharge and J-17 levels throughout the remainder of the 1950's drought. Simulation of J-17 levels for the 1952-56 period is improved using HDR/EUWD, rather than USGS, recharge estimates.

4.

3.

Consider calibration to the 1978-89 period rather than the 1947-59 period used previously. Recharge and pumpage estimates should be better during the 1978-89 period and the Edwards Aquifer experienced a comparable range of water levels (as measured at J-17). Alternatively, both the 1947-59 and 1978-89 periods (or the entire 1934-89 period) could be used for calibration.

Estimates of historical recharge which occurred in the Upper San Marcos River watershed could be improved by consideration of daily surface water runoff estimates which were manually removed from gaged records on the San Marcos River during the annual processing of San Marcos springflows. It is our understanding that these records may exist in the USGS archives.

Model Enhancements

5.

4.

- 1. Incorporate program code to facilitate easy consideration of multiple drought management plan triggers or activities. Enhance capabilities to consider drought triggers keyed to monitoring wells in addition to J-17 or to Comal and San Marcos Springs. Improve capabilities to simulate activities such as irrigation purchase ("dry year option") or reduced pumpage in specific use categories / geographic areas. GWSIM4 should be capable of simulating redistribution and/or reduction of pumpage (by category of use and geographic region) on a monthly timestep based on springflow or well level triggers.
- 2. Improve/automate geographic distribution of historical recharge. Consider that most upstream cells on streams crossing the outcrop will have greatest opportunity for recharge and transmit "rejected" recharge (recharge in excess of cell storage capacity in a given month) to downstream cell(s).
- 3. Improve ability to retrieve specific data of interest from output of GWSIM4 model.
 - Update head-discharge relationships for all springs in GWSIM4 model. Consider non-linear or piecewise linear relationships for estimation of spring discharge from head levels if appropriate based on observed data. Also, consider possibility of different head-discharge relationships for rising and falling aquifer conditions. Any updated relationships would be determined based on historical well level and springflow data and, if significantly different, would replace those presently in GWSIM4.
- 5. Refine and/or incorporate relationships in the GWSIM4 model to simulate Edwards Aquifer flux at Hueco Springs and along the Guadalupe River between Canyon Dam and New Braunfels.
- 6. Modify model grid to include cell(s) in the Onion Creek watershed based on recent studies by LBG-Guyton for EUWD.

Consider bad water line location modifications in Medina and/or Uvalde Counties in accordance with recent EUWD study. It is suggested that such modifications only be considered at this time if they will result in noticeably improved simulation results and will not alter existing grid size and shape.

7.

- 8. Consider modifications (to the extent possible) to reflect improved geologic mapping being developed by the USGS for Hays, Comal, and Bexar Counties.
- 9. Formalize carry-over storage ("rejected" recharge) in the simulation of enhancement projects to following month. Ultimately, this kind of information needs to be tied back into surface water models.
- 10. Confirm extended cessation of discharges from Leona Springs simulated by the GWSIM4 model for annual pumpage rates of 400,000 acft and 450,000 acft.
- 11. Incorporate program code to facilitate simulation of surface water imports for recharge enhancement.
- 12. Consider modifications to account for various estimates of interformational flux from the Trinity Aquifer.
- 13. If model recalibration and enhancements significantly improve performance, consider development of program code to facilitate automated computation of Edwards Aquifer "firm yield" subject to various springflow and/or well level constraints.
- 14. Consider new capability to initialize heads throughout the aquifer for any time during the historical record. In order to perform simulations to predict potential future water levels and springflows, an accurate set of initial heads for cells comprising the aquifer for a given starting time should be generated based on available data from observation wells.

4

Edwards Aquifer Recharge Enhancement Projects

ſ

-

-

[

ſ

I

[

T

I

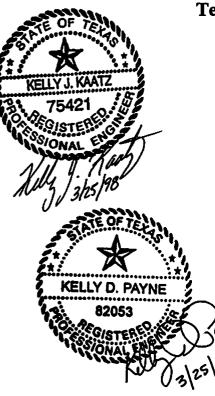
T

TRANS-TEXAS WATER PROGRAM WEST CENTRAL STUDY AREA

PHASE 2

GUADALUPE - SAN ANTONIO RIVER BASIN RECHARGE ENHANCEMENT STUDY FEASIBILITY ASSESSMENT

San Antonio River Authority San Antonio Water System Edwards Aquifer Authority Guadalupe-Blanco River Authority Lower Colorado River Authority Bexar Metropolitan Water District Nueces River Authority Canyon Lake Water Supply Corporation Bexar-Medina-Atascosa Counties WCID No. 1 Texas Natural Resource Conservation Commission Texas Parks and Wildlife Department Texas Water Development Board





Paul Price Associates, Inc. LBG-Guyton Associates Fugro-McClelland (SW), Inc.

March 1998





3.0 RECHARGE ENHANCEMENT PROGRAM DEVELOPMENT

A range of storage capacities was examined for each proposed recharge enhancement project (except the Northern Bexar / Medina County projects) in order to determine an optimum size. In determining the range of storage capacities to evaluate, consideration was given to several factors including watershed area, site topography, and known site constraints that would increase project costs, such as major road relocations and inundation of structures. Five different storage capacities were evaluated for each of the four major recharge projects. For the five smaller projects in Northern Bexar and Medina County, the recharge pool volumes were set equal to the 100-year flood volume computed for each site.

The optimum size storage capacity for each major project was selected on the basis of the minimum unit cost of recharge enhancement under long-term (1934-1989) average conditions. Applying this criteria, the smallest storage capacity evaluated at each of the major projects was determined to be the optimum size.

During the individual project evaluations, it became apparent that the unit cost of recharge enhancement at the Upper Blanco site is considerably more expensive than that for the Lower Blanco site. Although the topography of the Upper Blanco site is very favorable for construction of a dam, the amount of water that could be recharged via releases across the downstream recharge zone and diversion from the reservoir to the Upper San Marcos watershed structures was significantly less than recharge enhancement at the Lower Blanco site. This resulted in unit costs for recharge enhancement, under both average and drought conditions, that were significantly higher than unit costs at the Lower Blanco site for all storage capacities evaluated. Given this, the Upper Blanco site was eliminated from consideration in the development of the recharge enhancement program for the Guadalupe - San Antonio River Basin. It should be noted, however, that the Upper Blanco project may have indirect water supply benefits such as more definitive control (with respect to timing) of the water to be used for recharge enhancement.

3.1 Sizing of Projects in Guadalupe - San Antonio River Basin

On the basis of this study, the Cibolo Creek, Lower Blanco, and San Geronimo Creek recharge enhancement projects are believed to be ready to move forward to a preliminary design

Trans-Texas Water Program West Central Study Area and permitting phase at this time. The recommended size of each major project was determined by examining the unit cost of recharge enhancement under average conditions for each of the storage capacities evaluated. The sizing procedure began by selecting the storage capacity of each project having the lowest unit cost (i.e., optimum size) and continued by enlarging the projects up to the maximum storage capacity considered.

Table 3.1-1 illustrates this process. The Cibolo Creek project at its optimum size represents the lowest unit cost of recharge enhancement of the three (Upper Blanco excluded) major projects. The next most cost effective quantity of recharge enhancement is obtained by developing the Lower Blanco project at its optimum size. The third most cost effective increment of recharge enhancement is obtained by enlarging the storage capacity of the Cibolo Creek project from 1,000 to 5,000 acft. The San Geronimo Creek project at its optimum (smallest) size enters the program ranked fourth. The program development continues by evaluating the incremental cost to enlarge each project up to the maximum storage capacity considered for each of the projects.

Graphical presentations of the recharge program development are shown in Figures 3.1-1 and 3.1-2. The points on the graphs correspond to the unit or incremental cost rankings as presented in Table 3.1-1. A fairly well defined break point occurs in the program development process at the 11th ranked project. This point represents the Lower Blanco project developed to its full potential storage capacity of 50,000 acft. Beyond this point, the unit cost of recharge enhancement begins to increase sharply, as relatively small amounts of additional recharge enhancement are added to the program. Figure 3.1-2 illustrates that virtually no additional recharge enhancement during the 10-year drought period (1947-1956) is added beyond the 11th ranked project.

The 12th step in the program development represents enlarging the storage capacity at the Cibolo Creek project from 10,000 to 50,000 acft. Detailed geohydrological investigations will be necessary for this larger size to determine if the potential environmental and socioeconomic impacts to Bracken Bat Cave and Natural Bridge Caverns¹ are worth the relatively small

¹ Natural Bridge Caverns, Various letters to U.S. National Park Service and San Antonio River Authority, April 4, 1995 to April 2, 1996.

		Table 3				
		Guadalupe-San Ant				
	Rech	arge Enhancement F	rogram Develo			
				Recharge Enhancement (acft/yr)		
Cost Ranking ¹	Average Unit or Incremental Cost to Enlarge (S/acft/yr)	Project	Optimum or Enlarged Storage Capacity (acft)	Average Conditions	Drought Conditions	
1	80	Cibolo Creek	1,000	3,787	382	
2	104	Lower Blanco Subtotals	$\frac{3,500}{4,500}$	<u>22,129</u> 25,916	<u>9,789</u> 10,171	
3	120	Cibolo Creek Subtotals	$\frac{5,000}{8,500}$	$\frac{4,138}{30,054}$	10,721	
4	142	San Geronimo Subtotals	8, <u>850</u>	<u>2,375</u> 32,429	528 11,249	
5	193	San Geronimo Subtotals	$\frac{1,000}{9,500}$	32,934	11,351	
6	164	San Geronimo Subtotals	<u>3,500</u> 12,000	33, <mark>182</mark>	11,366	
7	196	Lower Blanco Subtotals	$\frac{10,000}{18,500}$	6,348 39,530	<u>3,471</u> 14,837	
8	183	Lower Blanco Subtotals	$\frac{17,500}{26,000}$	<u>5,078</u> 44,608	<u>2,225</u> 17,062	
9	83	Lower Blanco Subtotals	<u>35,000</u> 43,500	<u>9,349</u> 53,957	$\frac{3,807}{20,869}$	
10	201	Cibolo Subtotals	$\frac{10,000}{48,500}$	<u>1,808</u> 55,765	<u>553</u> 21,422	
11	230	Lower Blanco Subtotals	$\frac{50,000}{63,500}$	<u>6,862</u> 62,627	$\frac{3,198}{24,620}$	
12	288	Cibolo Creek Subtotals	<u>50,000</u> 103,500	<u>3,116</u> 65,734	25, 604	
13	720	Bexar/Medina Sites Subtotals	<u>12,409</u> 115,909	<u>2,429</u> 68,172	26,105	
.14	2,124	San Geronimo Subtotals	7,000 119,400	68,247	<u>6</u> 26,111	
15	31,897	San Geronimo Subtotals	$\frac{14,000}{126,409}$	68,275	26,1 <u>21</u>	

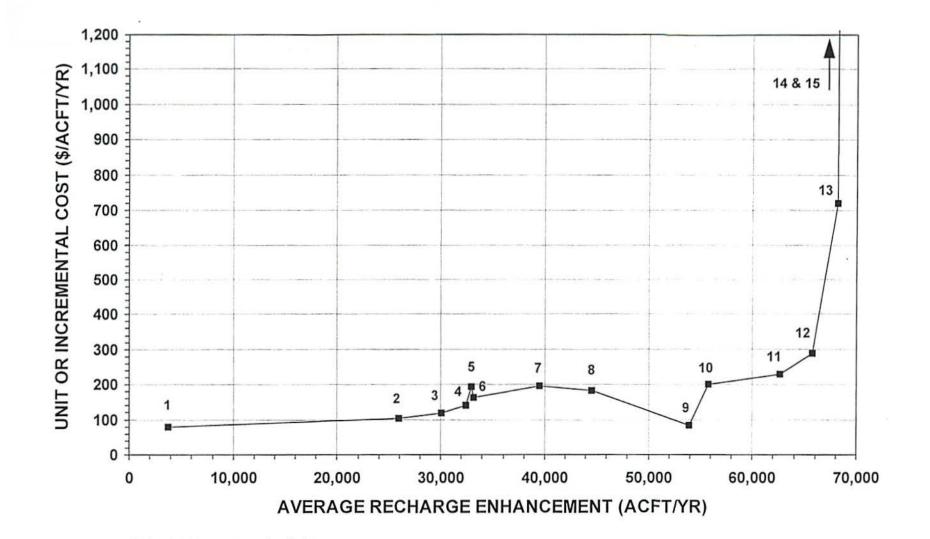
Trans-Texas Water Program West Central Study Area

Guadalupe - San Antonio River Basin Recharge Enhancement Study Feasibility Assessment

GUADALUPE - SAN ANTONIO RIVER BASIN RECHARGE ENHANCEMENT STUDY FEASIBILITY ASSESSMENT HR

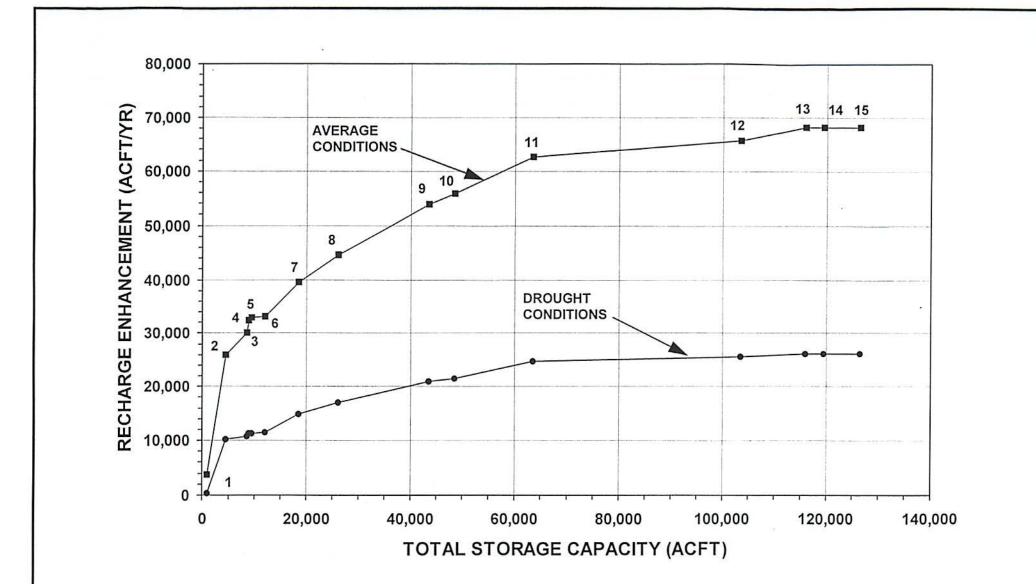
RECHARGE ENHANCEMENT PROGRAM DEVELOPMENT COST SUMMARY

TRANS TEXAS WATER PROGRAM / WEST CENTRAL STUDY AREA



HDR Engineering, Inc.

FIGURE 3.1-1



TRANS TEXAS WATER PROGRAM / WEST CENTRAL STUDY AREA

GUADALUPE - SAN ANTONIO RIVER BASIN RECHARGE ENHANCEMENT STUDY FEASIBILITY ASSESSMENT HR

RECHARGE ENHANCEMENT PROGRAM DEVELOPMENT STORAGE SUMMARY

HDR Engineering, Inc.

FIGURE 3.1-2

amounts of additional average and drought recharge enhancement obtained by enlarging the project. Other potential benefits, although not addressed by this study, may exist for an enlarged project. These may include flood control and use of the enlarged recharge pool as a discharge location for imported water.

The group of five smaller Northern Bexar / Medina County projects enters the program ranked 13th, with a unit cost for recharge enhancement of \$720/acft/yr under average conditions, as shown in Table 3.1-1. Although the cost of recharge enhancement appears to be very high for these smaller projects, other benefits such as flood control, may be derived from the development of these projects in the growing northwestern suburbs of San Antonio. These projects may also be utilized as discharge locations for water imported to enhance recharge and/or recirculation of Edwards Aquifer springflow.

3.2 Summary of Recommended Recharge Enhancement Program for Guadalupe - San Antonio River Basins (L-21)

The recommended recharge enhancement program is comprised of the Cibolo Creek project sized at 10,000 acft, Lower Blanco at 50,000 acft with diversion to the Upper San Marcos watershed flood retardation structures, and San Geronimo Creek at 3,500 acft. A summary of the recommended program is presented in Table 3.2-1. Development of this program would provide 62,627 acft/yr of recharge enhancement under average conditions at an average unit cost of \$135/acft/yr (\$0.41 per 1,000 gallons). Recharge enhancement under drought conditions would be 24,620 acft/yr at an average unit cost of \$344/acft/yr (\$1.06 per 1,000 gallons). The total capital cost of the recommended recharge enhancement program is estimated to be \$81.8 million and the total annual cost for this program would be about \$8.5 million.

A graph showing how the annual recharge to the Edwards Aquifer occurring in the Guadalupe - San Antonio River Basin would be affected by implementation of the recommended program is presented in Figure 3.2-1. This figure illustrates natural recharge to the Edwards Aquifer and recharge enhancement resulting from development of the recommended program. Recharge to the Guadalupe - San Antonio River Basin portion of the Edwards Aquifer would be increased by approximately 20 percent under average conditions and 16 percent under drought conditions with the implementation of the recommended recharge enhancement program.

Trans-Texas Water Program West Central Study Area

2

.....

and the second s

•

.....

 <u>....</u>

Table 3.2-1 Summary of Recommended Recharge Enhancement Program for Guadalupe-San Antonio River Basin									
					Average (Conditions	Drought Conditions		
Rank*	Project	Capacity (acft)	Surface Area (ac)	Annual Cost (\$)	Recharge Enhancement (acft/yr)	Cost/Unit Recharge Enhancement (\$/acft/yr)	Recharge Enhanceme nt (acft/yr)	Cost/Unit Recharge Enhancement (\$/acft/yr)	
1	Cibolo Creek	10,000	476	1,165,724	9,733	120	1,485	785	
2	Lower Blanco	50,000	1,408	6,830,020	49,766	137	22,490	304	
3	San Geronimo Total	<u>3,500</u> 63,500	<u>183</u> 2,067	<u>475,476</u> 8,471,220	<u>3,128</u> 62,627	152	<u>645</u> 24,620	737	
	Average	·		·		135		344	
*Rank is	based on cost/unit re	echarge enhance	ment for averag	e conditions.					

900,000 800,000 700,000 600,000 **RECHARGE (ACFT)** 500,000 400,000 300,000 200,000 100,000 0 1970 1982 1960 1962 1964 1966 1972 1976 1978 1984 1988 1936 1938 1940 1942 1946 1948 1950 1952 1954 1956 1958 1968 1974 1980 1986 1934 944 TIME (YEARS) □ NATURAL RECHARGE RECHARGE ENHANCEMENT W/ RECOMMENDED PROGRAM TRANS TEXAS WATER PROGRAM / WEST CENTRAL STUDY AREA **EDWARDS AQUIFER RECHARGE GUADALUPE-SAN ANTONIO GUADALUPE - SAN ANTONIO RIVER BASIN RIVER BASIN RECHARGE ENHANCEMENT STUDY FEASIBILITY ASSESSMENT**

HDR Engineering, Inc.

FIGURE 3.2-1

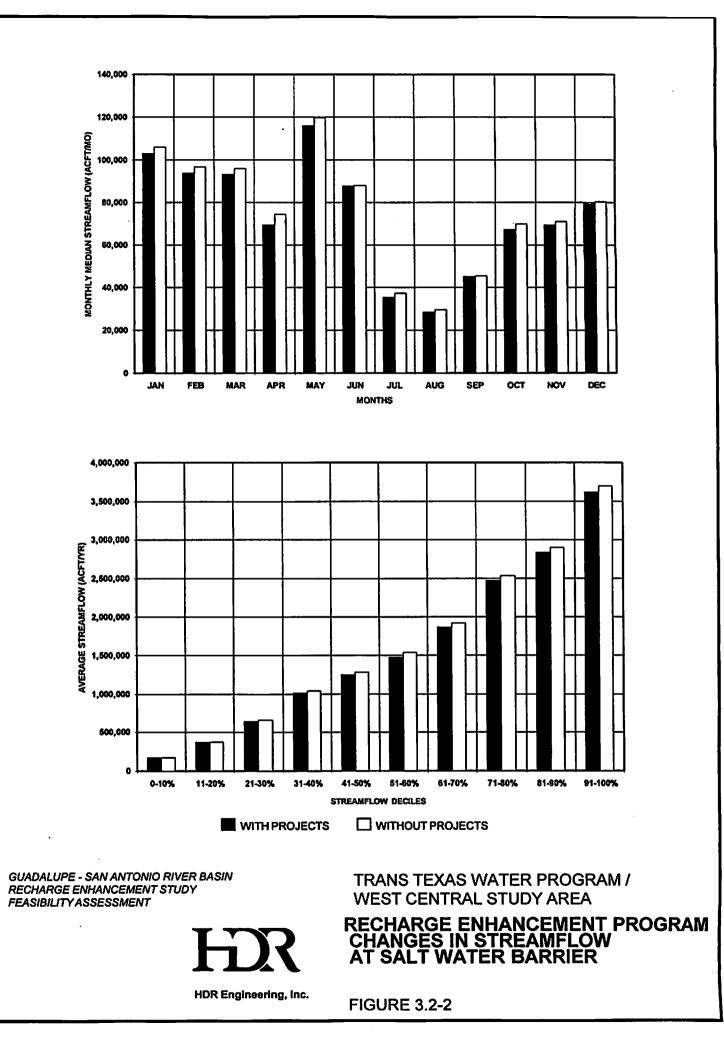
Ϋ́

Cumulative downstream impacts associated with the program are represented by changes in streamflow at the Saltwater Barrier, as presented in Figure 3.2-2. Based on the minimal reduction in estuarine inflow, potential impacts to fisheries harvest, salinity fluctuations, and nutrient/sediment loadings are likely to be insignificant as a result of development of the recommended recharge enhancement program in the Guadalupe - San Antonio River Basin. Long-term average annual streamflows at the Saltwater Barrier would decrease approximately 2.5 percent from 1,625,115 acft/yr without recharge enhancement to 1,585,088 acft/yr with the three recommended projects. This represents a maximum upper limit of impact, since enhanced springflows resulting from the additional recharge will reduce these impacts. Median monthly flow changes with the projects range from a maximum decrease due to the projects of 4,855 acft per month (7 percent) in April to a minimum decrease of 272 acft per month (0.3 percent) in June.

3.3 Combined Program for Nueces and Guadalupe - San Antonio River Basins (L-18A)

A recharge enhancement study for the Nueces River Basin was completed by the EUWD in June, 1994.² The recommended recharge enhancement program resulting from that study consisted of four projects, each constructed at its optimum size. These projects included, from east to west, the Lower Verde, Hondo, Sabinal, and Frio Projects. As discussed in Section 3.1 for the Cibolo Creek and Bexar/Medina County projects in the Guadalupe — San Antonio Basin, the recharge projects in the Nueces River Basin could be enlarged to obtain additional flood control benefits and/or to facilitate recharge of imported water. For comparison purposes in this study, capital costs for the recommended Nueces River Basin projects were updated from mid-1994 to the first quarter 1996 level using U.S. Bureau of Reclamation Construction Cost Indices (USBR CCI) for earth or concrete dams (as appropriate) and for secondary road relocations. Land acquisition costs were held constant and environmental mitigation costs were inflated by seven percent over the 21-month period. Total capital cost of the Nueces River Basin

² HDR Engineering, Inc., "Nueces River Basin Edwards Aquifer Recharge Enhancement Project, Phase IVA," Edwards Underground Water District, June, 1994.



ŀ.

recharge enhancement program is estimated to be \$60.0 million and the total annual cost for this program would be about \$7.0 million.

A summary of the recommended recharge enhancement program for the Nueces River Basin is presented in Table 3.3-1. Development of this program would provide 45,135 acft/yr of recharge enhancement under average conditions at an average unit cost of \$156/acft/yr (\$0.48 per 1,000 gallons). Recharge enhancement under drought conditions would be 9,250 acft/yr at an average unit cost of \$760/acft/yr (\$2.33 per 1,000 gallons). Costs to mitigate impacts to the Choke Canyon Reservoir / Lake Corpus Christi System yield and reductions in fresh water inflows to the Nueces Estuary were included in the development of project costs.

A combined recharge enhancement program for the Edwards Aquifer has been developed by ranking the recommended projects in the Nueces and Guadalupe - San Antonio River Basins based on the unit cost of recharge enhancement under average conditions. The combined recharge enhancement program is presented in Table 3.3-2. Graphical presentations of this program are shown in Figures 3.3-1 and 3.3-2. Development of this combined program could provide 107,762 acft/yr of recharge enhancement under average conditions at an average unit cost of \$144/acft/yr (\$0.44 per 1,000 gallons). Recharge enhancement under drought conditions would be 33,870 acft/yr at an average unit cost of \$458/acft/yr (\$1.41 per 1,000 gallons). The total capital cost of the combined Edwards Aquifer recharge enhancement program is estimated to be \$141.8 million and the total annual cost for this program would be about \$15.5 million.

As shown in Table 3.3-2, the Lower Blanco project represents a significant portion of the recharge enhancement under both long-term and drought average conditions. The calculation of potential recharge enhancement and, therefore, the unit cost of enhancement is a function of the natural percolation rate used for the recharge pool in the model. Detailed geologic and hydrogeologic investigations of the Lower Blanco reservoir area will be necessary to determine natural and expected recharge rates and the subsequent movement of ground water from the site. A similar conclusion was reached for the proposed Indian Creek project on the Nueces River in the 1994 Nueces River Basin recharge enhancement study.

 •

 •

Table 3.3-1 Summary of Recharge Enhancement Program for Nueces River Basin									
		Capacity ject (acft)	Surface Arca (ac)	Annual Cost (\$)	Average (Conditions	Drought Conditions		
Rank*	Project				Recharge Enhancement (acft/yr)	Cost/Unit Recharge Enhancement (\$/acft/yr)	Recharge Enhancement (acft/yr)	Cost/Unit Recharge Enhancement (\$/acft/yr)	
1	Lower Sabinal	8,750	454	1,420,829	16,442	86	2,358	603	
2	Lower Verde	3,600	334	647,148	4,850	133	1,719	376	
3	Lower Hondo	2,800	232	1,335,515	6,779	197	1,193	1,119	
4	Lower Frio	<u>17,500</u>	<u>1,099</u>	<u>3,628,170</u>	<u>17,064</u>	213	<u>3,980</u>	912	
	Total	32,650	2,119	7,031,662	45,135		9,250		
	Average					156		760	

3

 ġ,

.

<u>----</u>

Trans-Texas Water Progru West Central Study Area	
ram	

 .

	Table 3.3-2										
		Combir	hed Recharge	Annual Cost (\$)	nt Program for Average (Edwards Aquif	Drought Conditions				
·Rank*	Project	Capacity (acft)	Surface Area (ac)		Recharge Enhancement (acft/yr)	Cost/Unit Recharge Enhancement (\$/acft/yr)	Recharge Enhancement (acft/yr)	Cost/Unit Recharge Enhancement (S/acft/yr)			
1	Lower Sabinal	8,750	454	1,420,829	16,442	86	2,358	603			
2	Cibolo Creek	10,000	476	1,165,724	9,733	120	1,485	785			
3	Lower Verde	3,600	334	647,148	4,850	133	1,719	376			
4	Lower Blanco	50,000	1,408	6,830,020	49,766	137	22,490	304			
5	San Geronimo	3,500	183	475,476	3,128	152	645	737			
6	Lower Hondo	2,800	232	1,335,515	6,779	197	1,193	1,119			
7	Lower Frio	17,500	<u>1,099</u>	3,628,170	<u>17,064</u>	213	<u>3,980</u>	912			
	Total	96,150	4,186	15,502,882	107,762		33,870				
	Average					144		458			
*Rank is	based on cost/unit re	echarge enhance	ment for averag	e conditions.			I	I			

 ·----

 1

 - inter

~----

_

3-13

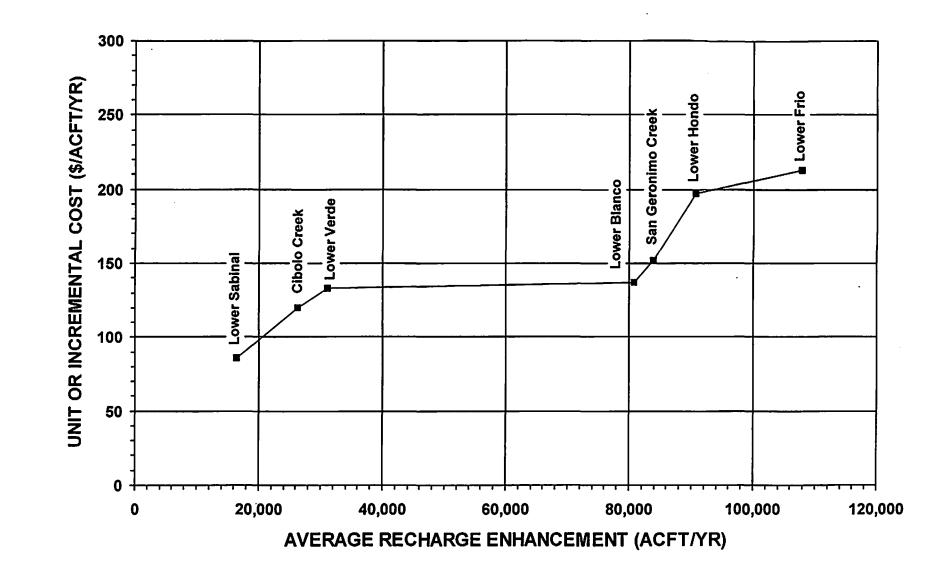
GUADALUPE - SAN ANTONIO RIVER BASIN RECHARGE ENHANCEMENT STUDY FEASIBILITY ASSESSMENT HR

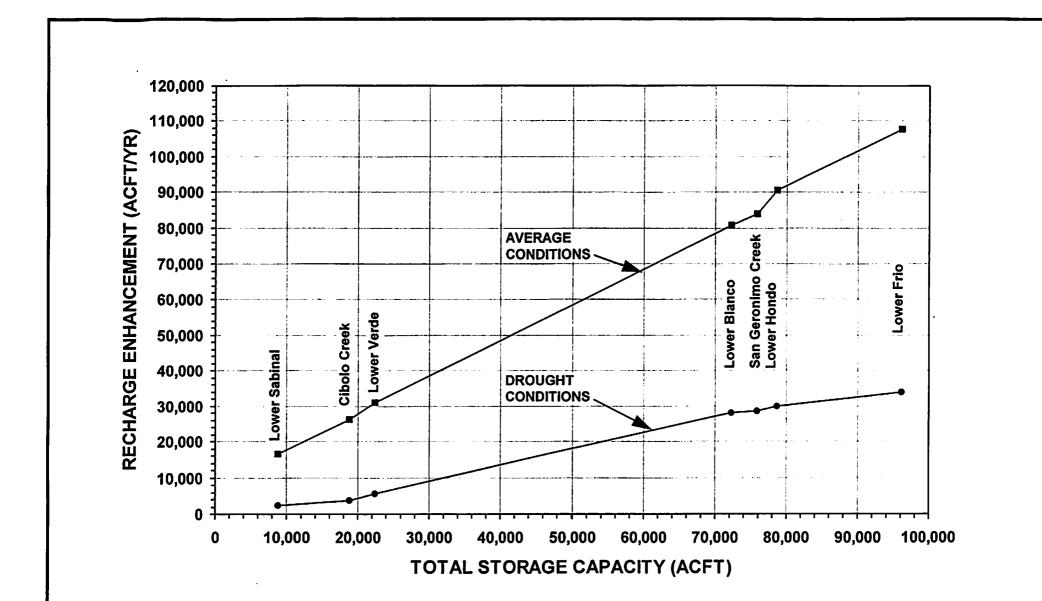
HDR Engineering, Inc.

RECHARGE ENHANCEMENT PROGRAM FOR COMBINED BASINS - COST SUMMARY

FIGURE 3.3-1

TRANS TEXAS WATER PROGRAM / WEST CENTRAL STUDY AREA





GUADALUPE - SAN ANTONIO RIVER BASIN RECHARGE ENHANCEMENT STUDY FEASIBILITY ASSESSMENT HR

WEST CENTRAL STUDY AREA
RECHARGE ENHANCEMENT

TRANS TEXAS WATER PROGRAM /

RECHARGE ENHANCEMENT PROGRAM FOR COMBINED BASINS - STORAGE SUMMARY

HDR Engineering, Inc.

FIGURE 3.3-2

Development of the Lower Blanco recharge project would likely result in sustained increases in flow from San Marcos Springs. These additional flows could be recaptured from the Guadalupe River below the San Marcos River confluence and diverted back to the Edwards Aquifer via a pipeline to the recharge zone. Conceptual studies on springflow recirculation (Alternatives L-22 and L-23) indicate that water diverted below Comal and or San Marcos Springs and introduced to the aquifer in northern Bexar County significantly benefits Comal Springs discharge thereby allowing more sustained pumpage during drought. Transferring water further west into Medina and/or Uvalde Counties could further elevate long-term storage levels in the aquifer, also increasing reliability of both pumpage and springflows during drought. Implementation of the recharge enhancement projects identified in this study is a key component in the overall management of the Edwards Aquifer.

To fully evaluate the potential benefits of implementing the recommended recharge program, it is recommended that the TWDB's GWSIM4 Model be used to evaluate the effects on increased aquifer pumpage and/or springflows. A systematic incremental analysis in which the enhanced recharge volumes produced by each recharge structure are incorporated into the groundwater model would clearly demonstrate the beneficial effects of each structure on aquifer pumpage and/or springflows. Additionally, this analysis should consider the combined benefits of implementing the recommended recharge program in combination with springflow recirculation.

Recirculation Concepts for Recharge Enhancement

ſ

[

T

-

ſ

-

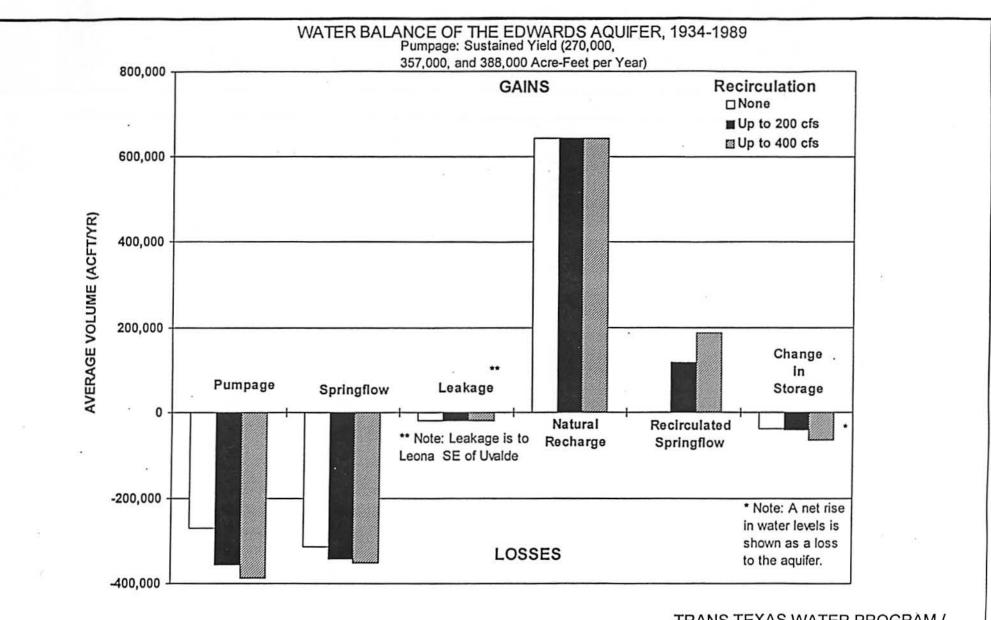
EDWARDS AQUIFER – SAN ANTONIO

SPRINGFLOW RECIRCULATION AND RECHARGE ENHANCEMENT March 9, 1998

What have we learned?

- Springflow Recirculation
 - 1. Northwestern Bexar County
 - For a recirculation rate of up to 200 cfs, an average of about 116,000 acft/yr would be available for recirculation;
 - About 75 percent of the recirculation (i.e., 87,000 ac-ft/yr) can be pumped from the aquifer and still sustain critical flows at Comal Springs;
 - Comal Springs begins to respond to recharge within a month or so and reaches a new equilibrium in about 10 years; and
 - Long-term Unit cost of water recharged to the aquifer is about \$260/acft/yr. The cost for water available for pumping is \$350/acft/yr.
 - For a "sustained yield" pumpage and 200 cfs recirculation to NW Bexar County, the average flow in the Guadalupe River decreased by 97 cfs. However, the decrease in flows during the drought were considerably less. (See attached graphs)
 - Water rights at the Saltwater Barrier generally decreased about 6,000 ac-ft/mo for the 200 cfs recirculation rate. (See Table) There is a very good potential of reducing or eliminating the impact by turning the recirculation 'OFF' during critical times.

- 2. Medina County
 - When operated in conjunction with northwestern Bexar County, recharge increases by about a third;
 - Additional Long-term Recirculation Volume is 69,000 acft/yr;
 - Additional Drought (1947-1956) Recirc. Vol. is 21,500 acft/yr;
 and
 - Additional "Sustained Yield" Pumpage is 31,000 acft/yr.
 - (Note: 31,000 acft/yr is 45 percent of the long term recirculation volume and 144 percent of the drought recirculation volume.)
 - Comal Springs response is very delayed, taking several decades for a new equilibrium to be established;
 - Incremental Unit cost (long-term) is about than five times more expensive than recharge to northwestern Bexar County; and
 - Recharge projects are more economical way to enhance recharge in Medina County.
- Recharge Enhancement
 See Table



TRANS TEXAS WATER PROGRAM / WEST CENTRAL STUDY AREA

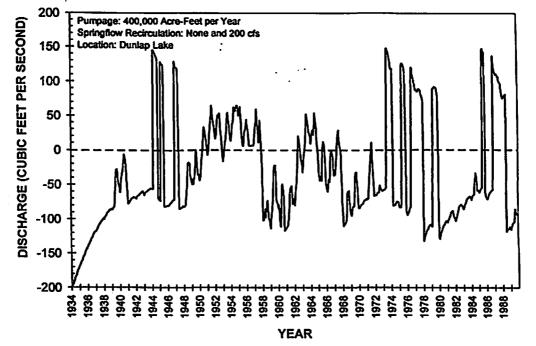
WATER BUDGET "SUSTAINED YIELD" PUMPAGE

CONCEPTUAL EVALUATION OF SPRINGFLOW RECIRCULATION

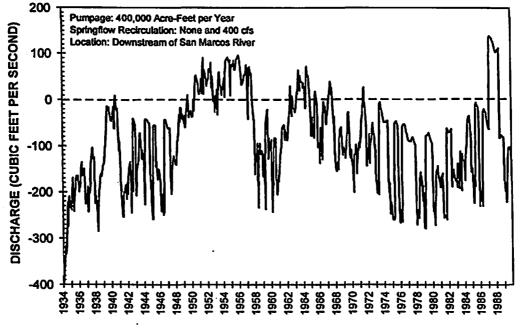
HDR Engineering, Inc.

FIGURE 3.2-2

GUADALUPE RIVER CHANGE IN FLOW AFTER DIVERSION



GUADALUPE RIVER CHANGE IN FLOW AFTER DIVERSION



YEAR

CONCEPTUAL EVALUATION OF SPRINGFLOW RECIRCULATION



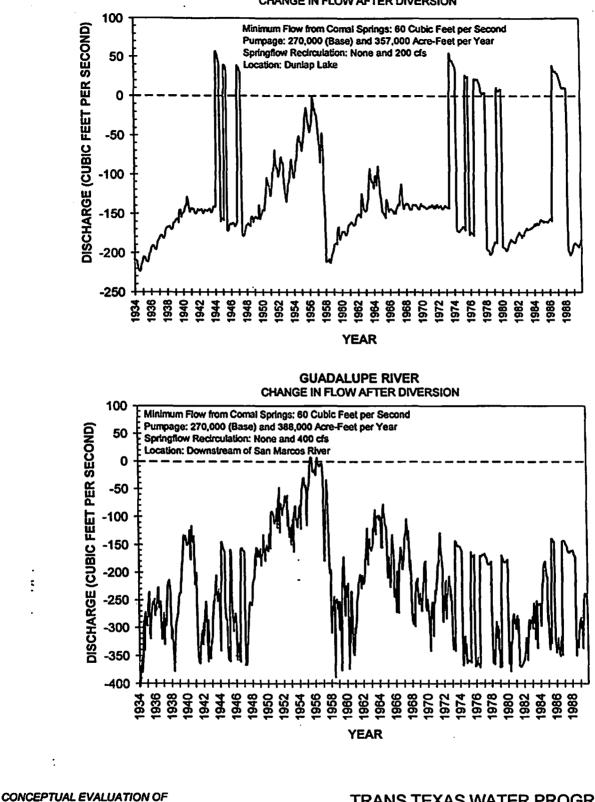
TRANS TEXAS WATER PROGRAM / WEST CENTRAL STUDY AREA

COMPUTED CHANGE IN FLOW GUADALUPE RIVER AFTER DIVERSION 400,000 ACFT/YR PUMPAGE

HDR Engineering, Inc.

FIGURE 3.1-5

GUADALUPE RIVER CHANGE IN FLOW AFTER DIVERSION



SPRINGFLOW RECIRCULATION

•

ξ

HDR Engineering, Inc.

TRANS TEXAS WATER PROGRAM / WEST CENTRAL STUDY AREA

COMPUTED CHANGE IN FLOW GUADALUPE RIVER AFTER DIVERSION "SUSTAINED YIELD" PUMPAGE

FIGURE 3.2-5

		Sustained I	eld" Pumpage	Viald in				
Shortage or Yield in ac-ft/yr								
Location	Total Water Rights (ac-ft)	Baseline no Recirculation	Up to 200 cfs Recirculation	۵	Up to 400 cfs Recirculation	Δ		
· ·	Long-Te	rm (1934-89) /	Average					
Guadalupe Riv., Victoria	23,806	0	0	0	0	0		
Guadalupe Riv., Saltwater Barrier	220,433	4,862	7,092	2,230	8,054	3,192		
San Antonio Riv., Falls City	9,311	0	0	0	0	0		
	Droug	ht (1947-56) A	verage ·					
Guadalupe Riv., Victoria	23,806	0	0	0	0	0		
Guadalupe Riv., Saltwater Barrier	220,433	18,887	23,789	4,901	24,112	5,225		
San Antonio Riv., Falls City	9,311	0	0	0	0	0		
Canyon Lake firm yield	1	87,124	86,492	-632	86,253	-871		

•

.

.

•

- - -----

.

·.

•

•

.

2

	·			Table	e 3.3-2					
Combined Recharge Enhancement Program for Edwards Aquifier										
				Annual Cost (\$)	Average (Conditions	Drought Conditions			
Rank*	Project	Capacity (acft)	Surface Area (ac)		Recharge Enhancement (acft/yr)	Cost/Unit Recharge Enhancement (S/acft/yr)	Recharge Enhancement (acft/yr)	Cost/Unit Recharge Enhancement (S/acft/yr)		
1	Lower Sabinal	8,750	454	1,420,829	16,442	86	2,358	603		
2	Cibolo Creek	10,000	476	1,165,724	9,733	120	1,485	785		
3	Lower Verde	3,600	334	647,148	4,850	133	1,719	376		
4	Lower Blanco	50,000	1,408	6,830,020	49,766	137	22,490	304		
5	San Geronimo	3,500	183	475,476	3,128	152	645	737		
6	Lower Hondo	2,800	232	1,335,515	6,779	197	1,193	1,119		
7	Lower Frio	<u>17,500</u>	. <u>1.099</u>	<u>3,628,170</u>	<u>17,064</u>	213	<u>3,980</u>	912		
	Total	96,150	4,186	15,502,882	107,762		33,870			
	Average					144		458		
Rank is	based on cost/unit rec	harge enhancem	ent for average c	onditions.		······································	· · · · · · · · · · · · · · · · · ·			

2

Estimated "Sustained Yield" Pumpage: Insufficient Information. Spr. Recir 116,000 260

					Average (Conditions	Drought Conditions		
Rank*	Project	Capacity (acft)	Surface Area (ac)	[•] Annual Cost (\$)	Recharge Enhancement (acft/yr)	Cost/Unit Recharge Enhancement (\$/acft/yr)	Recharge Enhancement (acft/yr)	Cost/Unit Recharge Enhancement (\$/acft/yr)	
1	Lower Sabinal	8,750	454	1,420,829	16,442	86	2,358	603	
2	Lower Verde	3,600	334	647,148	4,850	133	1,719	376	
3	Lower Hondo	2,800	232	1,335,515	6,779	197	1,193	1,119	
4	Lower Frio	<u>17.500</u>	<u>1.099</u>	3.628.170	<u>17.064</u>	213	<u>3,980</u>	912	
	Total	32,650	2,119	7,031,662	45,135		9,250		
	Average		•			156		760	
•Rank is	based on cost/unit rec	harge enhancem	ent for average c	onditions.	<u></u>				

1

2

(····

1

يبنيا

Notes: 11 45% of Total 12 144% of Total

2

3

••

Where do we go from here?

- To more fully evaluate the potential benefits of springflow recirculation, it is recommended that the current version of GWSIM4 be improved to more accurately evaluate potential and recommended springflow recirculation and recharge enhancement projects. These improvements should include:
 - the ability to easily modify starting head conditions within the model,
 - a reevaluation of the head-discharge relationships at each spring, especially at San Antonio, San Pedro, and Leona Springs,
 - a consideration of discharge from Hueco Springs and any recharge from the Guadalupe River, and
 - a consideration of recharge coming from Onion Creek which may improve simulations at San Marcos Springs. Consider GWSIM4 "improvement" of springflow discharge and water levels, especially in the San Antonio area. This would include:
- After GWSIM4 is improved, it is recommended that the following analysis be performed to fully evaluate the benefits of the recharge enhancement projects on the basis of "sustained yields" and unit cost of increased "sustained yields" both with and without springflow recirculation.
 - Use GWSIM4 to determine in a systematic manner "sustained yield" pumpage and associated unit costs for individual or groups of recommended recharge projects. This would be done initially without recirculation;
 - Use GWSIM4 to detemine optimum recirculation rate from Lake Dunlap with recommended recharge projects in place and determine "sustained yield" and unit costs for a range of recirculation rates. Consider adding other water sources, i.e., unappropriated water, unutilized water rights, or purchased water rights at Lake Dunlap. Also, consider the water supply benefits and costs of extending the recirculation pipeline to Medina Lake on both aquifer yield and reservoir yield. (Note: This analysis is intended to determine the upper limit of

aquifer pumpage for the combined effects of multiple recharge projects and water sources.)

- Determine optimum combination of recharge projects and recirculation rate by a systematic elimination of selected recharge projects to determine increased "sustained yield" and unit costs with recirculation in place; and
- Recommend optimum system and consider institutional and permitting issues associated with implementation to allow for pumping and springflow benefits to be fully realized.

General Questions and Comments

- Can surplus springflow recirculation be spilled into Medina Lake with credit for some of the water becoming recharge to the Edwards and some being withdrawn from Medina River?
- Can recirculation be turned 'OFF' during critical times to meet downstream senior water rights in the Guadalupe River?
- Can recirculated springflow in combination with other water sources be treated and delivered directly to municipal distribution system during high demand periods (summer) or when aquifer storage is 'full'?
- What is the most efficient way to recharge the recirculated springflow?
- Will injection wells be needed as a backup if target streams reject recharge?
- What are some of the other beneficial uses of recharge facilities?
- How can a recharge project in the Blanco River be used to benefit water users in the San Antonio area? For example, can enhanced springflow help ensure 100 cfs minimum at San Marcos Springs and/or potentially help mitigate reduced flows on the Guadalupe River caused by recirculation from Lake Dunlap?